STORM WATER MANAGEMENT PLAN FOR PALMA DE LA REINA BY NEWPORT PACIFIC

L-14372

Development Consisting of Commercial and Residential Buildings And Private Parking Lot

> Prepared By: San Dieguito Engineering, Inc. 4407 Manchester Ave., Suite 105 Encinitas, CA 92024

> > Report Prepared By:

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Date: February 19, 2004 Revised: February 28, 2007 Revised: May 28, 2008 SDE Job No. 4200.02

Storm Water Management Plan For Priority Projects (Major SWMP)

The Major Stormwater Management Plan (Major SWMP) must be completed in its entirety and accompany applications to the County for a permit or approval associated with certain types of development projects. To determine whether your project is required to submit a Major or Minor SWMP, please reference the County's Stormwater Intake Form for Development Projects.

Project Name:	Newport Pacific Palma De La Reina
Permit Number (Land Development Projects):	L-14372
Work Authorization Number (CIP only):	N/A
Applicant:	San Dieguito Engineering Inc.
Applicant's Address:	1407 Manchester Ave. Ste. 105, Encinitas, CA, 92024
Plan Prepare By (Leave blank if same as applicant):	
Date:	2/19/2004
Revision Date (If applicable):	

The County of San Diego Watershed Protection, Storm Water Management, and Discharge Control Ordinance (WPO) (Ordinance No. 9424) requires all applications for a permit or approval associated with a Land Disturbance Activity to be accompanied by a Storm Water Management Plan (SWMP) (section 67.806.b). The purpose of the SWMP is to describe how the project will minimize the short and long-term impacts on receiving water quality. Projects that meet the criteria for a priority development project are required to prepare a Major SWMP.

Since the SWMP is a living document, revisions may be necessary during various stages of approval by the County. Please provide the approval information requested below.

Project Stages		e SWMP visions?	If YES, Provide Revision Date
	YES	NO	Revision Date
L-Grading Permit (L-14372)		No	

Instructions for a Major SWMP can be downloaded at http://www.co.san-diego.ca.us/dpw/stormwater/susmp.html.

Completion of the following checklists and attachments will fulfill the requirements of a Major SWMP for the project listed above.

PROJECT DESCRIPTION

Please provide a brief description of the project in the following box. Please include:

- Project Location
- Project Description
- Physical Features (Topography)
- Surrounding Land Use
- Proposed Project Land Use

• Location of dry weather flows (year-round flows in streams, or creeks) within project limits, if applicable.

The 4.31 acre (gross) project is located in the Whispering Palms area of the County of San Diego, at the corner of Via De La Valle and Cancha De Golf (See Attachment A). This project will consist of construction of one office building, two retail buildings for commercial use, and nine buildings containing fifty-four apartments. In order to service the tenants of these buildings, two hundred and twenty parking spaces will be provided on-site. The parking spaces will have access from Via De La Valle, Cancha De Golf and Via De Las Palmas. Assumptions have been made regarding the proposed development, as required, based on the land usage in the surrounding area (Reference Section 1.1). The surrounding area is used for mixed commercial and single family residential purposes.
purposes.

PRIORITY DEVELOPMENT PROJECT DETERMINATION

Please check the box that best describes the project. Does the project meet one of the following criteria?

Table 1

PRIORITY DEVELOPMENT PROJECT	YES	NO
Redevelopment that creates or adds at least 5,000 net square feet of additional impervious surface area	x	
Residential development of more than 10 units	X	
Commercial developments with a land area for development of greater		· · ·
than 1 acre		X
Heavy industrial development with a land area for development of greater than 1 acre		Х
Automotive repair shop(s)		Х
Restaurants, where the land area for development is greater than 5,000 square feet		Х
Hillside development, in an area with known erosive soil conditions,		
where there will be grading on any natural slope that is twenty-five percent or greater, if the development creates 5,000 square feet or more of		Х
impervious surface		<u>-</u> .
Environmentally Sensitive Areas (ESA): All development located within or directly adjacent to or discharging directly to an ESA (where discharges from the development or redevelopment will enter receiving waters within the ESA), which either creates 2,500 square feet of impervious surface on a proposed project site or increases the area of imperviousness of a proposed project site to 10% or more of its naturally occurring condition. "Directly adjacent" means situated within 200 feet of the ESA. "Discharging directly to" means outflow from a drainage conveyance system that is composed entirely of flows from the subject development or redevelopment site, and not commingled with flows from adjacent lands.		х
Parking Lots 5,000 square feet or more or with 15 parking spaces or more and potentially exposed to urban runoff	×	
Streets, roads, highways, and freeways which would create a new paved surface that is 5,000 square feet or greater		×
Retail Gasoline Outlets (RGO) that meet the following criteria: (a) 5,000 square feet or more or (b) a projected Average Daily Traffic (ADT) of 100 or more vehicles per day.		Х

Limited Exclusion: Trenching and resurfacing work associated with utility projects are not considered Priority Development Projects. Parking lots, buildings and other structures associated with utility projects are subject to the WPO requirements if one or more of the criteria above are met.

If you answered **NO** to all the questions, then **STOP**. Please complete a Minor SWMP for your project.

If you answered YES to any of the questions, please continue.

HYDROMODIFICATION DETERMINATION

The following questions provide a guide to collecting information relevant to hydromodification management issues.

Table 2

	QUESTIONS	YES	NO	Information
1.	Will the proposed project disturb 50 or			If YES, continue to 2.
	more acres of land? (Including all phases		X	If NO, go to 6.
	of development)			
2.	Would the project site discharge directly			If NO, continue to 3.
	into channels that are concrete-lined or			If YES, go to 6.
	significantly hardened such as with rip-			
	rap, sackcrete, etc, downstream to their			
	outfall into bays or the ocean?			
3.	Would the project site discharge directly			If NO, continue to 4.
	into underground storm drains			If YES, go to 6.
	discharging directly to bays or the ocean?			
4.	Would the project site discharge directly			If NO, continue to 5.
	to a channel (lined or un-lined) and the			If YES, go to 6.
	combined impervious surfaces			
į	downstream from the project site to			
	discharge at the ocean or bay are 70% or			
	greater?			
5.	Project is required to manage			Hydromodification
	hydromodification impacts.			Management Required
				as described in Section
		13.1.1.1		67.812 b(4) of the
				WPO
6.	Project is not required to manage			Hydromodification
	hydromodification impacts.		48 77	Exempt. Keep on file.

An exemption is potentially available for projects that are required (No. 5. in Table 2 above) to manage hydromodification impacts: The project proponent may conduct an independent geomorphic study to determine the project's full hydromodification impact. The study must incorporate sediment transport modeling across the range of geomorphically-significant flows and demonstrate to the County's satisfaction that the project flows and sediment reductions will not detrimentally affect the receiving water to qualify for the exemption.

STORMWATER QUALITY DETERMINATION

The following questions provide a guide to collecting information relevant to project stormwater quality issues. Please provide the following information in a printed report accompanying this form.

Table 3

	QUESTIONS	COMPLETED	NA
1.	Describe the topography of the project area.	FLAT	
2.	Describe the local land use within the project area and adjacent areas.	MIXED USE	
3.	Evaluate the presence of dry weather flow.	NONE	
4.	Determine the receiving waters that may be affected by the project throughout all phases of development (i.e., construction, maintenance and operation).	COMPLETED	
5.	For the project limits, list the 303(d) impaired receiving water bodies and their constituents of concern.	COMPLETED	
6.	Determine if there are any High Risk Areas (which is defined by the presence of municipal or domestic water supply reservoirs or groundwater percolation facilities) within the project limits.	COMPLETED	
7.	Determine the Regional Board special requirements, including TMDLs, effluent limits, etc.	COMPLETED	
8.	Determine the general climate of the project area. Identify annual rainfall and rainfall intensity curves.	COMPLETED	
9.	If considering Treatment BMPs, determine the soil classification, permeability, erodibility, and depth to groundwater.	COMPLETED	
10.	Determine contaminated or hazardous soils within the project area.	COMPLETED	

TREATMENT BMPs DETERMINATION

Complete the checklist below to determine if Treatment Best Management Practices (BMPs) are required for the project.

Table 4

No.	CRITERIA	YES	NO	INFORMATION
1.	Is this an emergency project		Х	If YES, go to 6. If NO, continue to 2.
2.	Have TMDLs been established for surface waters within the project limit?	х		If YES, go to 5. If NO, continue to 3.
3.	Will the project directly discharge to a 303(d) impaired receiving water body?			If YES, go to 5. If NO, continue to 4.
4.	Is this project within the environmentally sensitive areas as defined on the maps in Appendix A of the County of San Diego Standard Urban Storm Water Mitigation Plan for Land Development and Public Improvement Projects?			If YES, continue to 5. If NO, go to 6.
5.	Provide Treatment BMPs for the project.	х		If YES, go to 7.
6.	Project is not required to provide Treatment BMPs			Document for Project Files by referencing this checklist.
7.	End			

Now that the need for a treatment BMPs has been determined, other information is required to complete the SWMP.

WATERSHED

Please check the watershed(s) for the project.

San Juan 901	☐ Santa Margarita 902	☐ San Luis Rey 903	☐ Carlsbad 904
■ San Dieguito 905	Penasquitos 906	☐ San Diego 907	Sweetwater 909
□Otay 910	☐Tijuana 911	☐ Whitewater 719	Clark 720
☐ West Salton 721	Anza Borrego 722	☐ Imperial 723	

Please provide the hydrologic sub-area and number(s)

Number	Name
905.11	RANCHO SANTA FE

Please provide the beneficial uses for Inland Surface Waters and Ground Waters. Beneficial Uses can be obtained from the Water Quality Control Plan for the San Diego Basin, which is available at the Regional Board office or at http://www.swrcb.ca.gov/rwqcb9/programs/basinplan.html.

SURFACE WATERS	Hydrologic Unit Basin Number	MUN	AGR	IND	PROC	GWR	FRESH	POW	REC1	REC2	BIOL	WARM	COLD	WILD	RARE	SPWN
Inland Surface Waters	905.11	*	0	0					Х	Х		X		Х		
	_										_					
Ground Waters	N/A															

* Excepted from Municipal

X Existing Beneficial Use

0 Potential Beneficial Use

POLLUTANTS OF CONCERN

Using Table 5, identify pollutants that are anticipated to be generated from the proposed priority project categories. Pollutants associated with any hazardous material sites that have been remediated or are not threatened by the proposed project are not considered a pollutant of concern.

Table 5. Anticipated and Potential Pollutants Generated by Land Use Type

. " .		•		General P	ollutant	Categories	-		
PDP Categories	Sediments Nutrients Heavy Organic Compounds		_	Trash & Debris	Oxygen Demanding Substances	Oil & Grease	Bacteria & Viruses	Pesticides	
Detached Residential Development	X	X			Х	X	X	X	X
Attached Residential Development	X	X			X	P ⁽¹⁾	P ⁽²⁾	P	X
Commercial Development 1 acre or greater	P ⁽¹⁾	P ⁽¹⁾		P ⁽²⁾	X	P ⁽⁵⁾	X	P ⁽³⁾	P ⁽⁵⁾
Heavy industry /industrial development	X		X	X	X	Х	X		
Automotive Repair Shops			X	$X^{(4)(5)}$	X		X		
Restaurants				-	X	X	X	X	
Hillside Development >5,000 ft ²	X	X			Х	X	X		X
Parking Lots	$\mathbf{P}^{(1)}$	$P^{(1)}$	X		X	$\mathbf{P}^{(1)}$	X		$P^{(1)}$
Retail Gasoline Outlets			X	X	X	X	X		
Streets, Highways & Freeways	X	$\mathbf{P}^{(1)}$	X	X ⁽⁴⁾	X	P ⁽⁵⁾	X		

X = anticipated

P = potential

- (1) A potential pollutant if landscaping exists on-site.
- (2) A potential pollutant if the project includes uncovered parking areas.
- (3) A potential pollutant if land use involves food or animal waste products.
- (4) Including petroleum hydrocarbons.
- (5) Including solvents.

Note: If other monitoring data that is relevant to the project is available. Please include as Attachment C.

CONSTRUCTION BMPs

Please check the construction BMPs that may be implemented during construction of the project. The applicant will be responsible for the placement and maintenance of the BMPs incorporated into the final project design.

X	Silt Fence		Desilting Basin						
×	Fiber Rolls	×	Gravel Bag Berm						
×	Street Sweeping and Vacuuming		Sandbag Barrier						
×	Storm Drain Inlet Protection	×	Material Delivery and Storage						
X	Stockpile Management	×	Spill Prevention and Control						
×	Solid Waste Management	×	Concrete Waste Management						
×	Stabilized Construction Entrance/Exit	×	Water Conservation Practices						
	Dewatering Operations	×	Paving and Grinding Operations						
×	Vehicle and Equipment Maintenance								
×	Any minor slopes created incidental to construction and not subject to a major or minor grading permit shall be protected by covering with plastic or tarp prior to a rain event, and shall have vegetative cover reestablished within 180 days of completion of								

the slope and prior to final building approval.

EXCEPTIONAL THREAT TO WATER QUALITY DETERMINATION

Complete the checklist below to determine if a proposed project will pose an "exceptional threat to water quality," and therefore require Advanced Treatment Best Management Practices.

Table 6

No.	CRITERIA	YES	NO	INFORMATION
1.	Is all or part of the proposed project site within 200 feet of waters named on the Clean Water Act (CWA) Section 303(d) list of Water Quality Limited Segments as impaired for sedimentation and/or turbidity? Current 303d list may be obtained from the following site: http://www.swrcb.ca.gov/tmdl/docs/303dlists2006/approved/r9_06_303d_reqt_mdls.pdf		×	If YES, continue to 2. If NO, go to 5.
2.	Will the project disturb more than 5 acres, including all phases of the development?			If YES, continue to 3. If NO, go to 5.
3.	Will the project disturb slopes that are steeper than 4:1 (horizontal: vertical) with at least 10 feet of relief, and that drain toward the 303(d) listed receiving water for sedimentation and/or turbidity?			If YES, continue to 4. If NO, go to 5.
4.	Will the project disturb soils with a predominance of USDA-NRCS Erosion factors k _f greater than or equal to 0.4?			If YES, continue to 6. If NO, go to 5.
5.	Project is not required to use Advanced Treatment BMPs.	x		Document for Project Files by referencing this checklist.
6.	Project poses an "exceptional threat to water quality" and is required to use Advanced Treatment BMPs.			Advanced Treatment BMPs must be consistent with WPO section 67.811(b)(20)(D) performance criteria

Exemption potentially available for projects that require advanced treatment: Project proponent may perform a Revised Universal Soil Loss Equation, Version 2 (RUSLE 2), Modified Universal Soil Loss Equation (MUSLE), or similar analysis that shows to the County official's satisfaction that advanced treatment is not required

Now that the need for treatment BMPs has been determined, other information is needed to complete the SWMP.

SITE DESIGN

To minimize stormwater impacts, site design measures must be addressed. The following checklist provides options for avoiding or reducing potential impacts during project planning. If YES is checked, it is assumed that the measure was used for this project.

Table 7

		OPTIONS	YES	NO	N/A
1.	to ave	the project been located and road improvements aligned bid or minimize impacts to receiving waters or to use the preservation of critical (or problematic) areas as floodplains, steep slopes, wetlands, and areas with we or unstable soil conditions?	x		
2.	Is the	project designed to minimize impervious footprint?	Х		
3.	Is the	project conserving natural areas where feasible?	Х		
4.	Wher sidew adjace	х			
5.	or loc	padway projects, are structures and bridges be designed atted to reduce work in live streams and minimize ruction impacts?			х
6.		ny of the following methods be utilized to minimize on from slopes:			
	6.a.	Disturbing existing slopes only when necessary?	Х		
	6.b.	Minimize cut and fill areas to reduce slope lengths?	Х		
	6.c.	Incorporating retaining walls to reduce steepness of slopes or to shorten slopes?	Х		
	6.d.	Providing benches or terraces on high cut and fill slopes to reduce concentration of flows?	х		
	6.e.	Rounding and shaping slopes to reduce concentrated flow?	Х		
	6.f.	Collecting concentrated flows in stabilized drains and channels?	Х		

LOW IMPACT DEVELOPMENT (LID)

Each numbered item below is a LID requirement of the WPO. Please check the box(s) under each number that best describes the Low Impact Development BMP(s) selected for this project.

Table 8

1. Conserve natural Areas, Soils, and Vegetation-County LID Handbook 2.2.1
☐ Preserve well draining soils (Type A or B)
☐ Preserve Significant Trees
Other. Description:
1. Not feasible. State Reason: ALL OF THE PROPERTY IS BEING DEVELOPED.
2. Minimize Disturbance to Natural Drainages-County LID Handbook 2.2.2
☐ Set-back development envelope from drainages
Restrict heavy construction equipment access to planned green/open
space areas
Other. Description:
☑ 2. Not feasible. State Reason:
NO NATURAL DRAINAGE AREAS NEARBY. EXISTING STORMDRAIN FACILITIES IMMEDIATELY OFFSITE.
3. Minimize and Disconnect Impervious Surfaces (see 5) -County LID Handbook 2.2.3
Clustered Lot Design
Items checked in 5?
Other. Description:
☐ 3. Not feasible. State Reason:
4. Minimize Soil Compaction-County LID Handbook 2.2.4
Restrict heavy construction equipment access to planned green/open
space areas
Re-till soils compacted by construction vehicles/equipment
☑ Collect & re-use upper soil layers of development site containing organic
materials
Other. Description:
4. Not feasible. State Reason:
5. Drain Runoff from Impervious Surfaces to Pervious Areas-County LID Handbook 2.2.5

LID	Street & Road Design
	Curb-cuts to landscaping
	Rural Swales
	Concave Median
	Cul-de-sac Landscaping Design
RUN-OFF FROM PA	Other. Description: /ED SURFACES DRAINS TO BIO-SWALE PRIOR TO LEAVING SITE.
LID	Parking Lot Design
	Permeable Pavements
	Curb-cuts to landscaping
RUN-OFF FROM PAV	Other. Description: VED SURFACES DRAINS TO BIO-SWALE PRIOR TO LEAVING SITE.
LID	Driveway, Sidewalk, Bike-path Design
	Permeable Pavements
	Pitch pavements toward landscaping
RUN-OFF FROM PA\	Other. Description: /ED SURFACES DRAINS TO BIO-SWALE PRIOR TO LEAVING SITE.
LID	Building Design
	Cisterns & Rain Barrels
	Downspout to swale
	Vegetated Roofs
×	Other. Description:
RUN-OFF FROM PAY	VED SURFACES DRAINS TO BIO-SWALE PRIOR TO LEAVING SITE.
LID	Landscaping Design
×	Soil Amendments
X	Reuse of Native Soils
×	Smart Irrigation Systems
	Street Trees
FOUNDATION PLAN	Other. Description:
□ 5. No	t feasible. State Reason:

CHANNELS & DRAINAGES

Complete the following checklist to determine if the project includes work in channels.

Table 9

No.	CRITERIA	YES	NO	N/A	COMMENTS
1.	Will the project include work in channels?		х		If YES go to 2
			^		If NO go to 13.
2.	Will the project increase velocity or				If YES go to 6.
	volume of downstream flow?				
3.	Will the project discharge to unlined				If YES go to. 6.
	channels?				
4.	Will the project increase potential	•			If YES go to 6.
	sediment load of downstream flow?				
5.	Will the project encroach, cross, realign,				If YES go to 8.
	or cause other hydraulic changes to a	į.			
	stream that may affect downstream				
	channel stability?				
6.	Review channel lining materials and				Continue to 7.
	design for stream bank erosion.				
7.	Consider channel erosion control measures				Continue to 8.
	within the project limits as well as				
	downstream. Consider scour velocity.		<u>_</u>		
8.	Include, where appropriate, energy				Continue to 9.
	dissipation devices at culverts.				
9.	Ensure all transitions between culvert				Continue to 10.
	outlets/headwalls/wingwalls and channels				
	are smooth to reduce turbulence and scour.				
10.	Include, if appropriate, detention facilities				
	to reduce peak discharges.				
1,	"Hardening" natural downstream areas to				Continue to 12.
11.	prevent erosion is not an acceptable				
	technique for protecting channel slopes,				
	unless pre-development conditions are				
	determined to be so erosive that hardening				
	would be required even in the absence of				
12	the proposed development.				Cantinua ta 12
12.	Provide other design principles that are				Continue to 13.
12	comparable and equally effective.				
13.	End				

SOURCE CONTROL

Please complete the following checklist for Source Control BMPs. If the BMP is not applicable for this project, then check N/A only at the main category.

Table 10

1.a. All storm drain inlets and catch basins within the project area shall have a stencil or tile placed with prohibitive language (such as: "NO DUMPING – DRAINS TO") and/or graphical icons to discourage illegal dumping. 1.b. Signs and prohibitive language and/or graphical icons, which prohibit illegal dumping, must be posted at public access points along channels and creeks within the project area. 2. Design Outdoors Material Storage Areas to Reduce Pollution Introduction 2.a. This is a detached single-family residential project. Therefore, personal storage areas are exempt from this requirement. 2.b. Hazardous materials with the potential to contaminate urban runoff shall either be: (1) placed in an enclosure such as, but not limited to, a cabinet, shed, or similar structure that prevents contact with runoff or spillage to the storm water conveyance system; or (2) protected by secondary containment structures such as berms, dikes, or curbs. 2.c. The storage area shall be paved and sufficiently impervious to contain leaks and spills. 2.d. The storage area shall have a roof or awning to minimize direct precipitation within the secondary containment area. 3. Design Trash Storage Areas to Reduce Pollution Introduction 3.a. Paved with an impervious surface, designed not to allow run-on from adjoining areas, screened or walled to prevent off-site transport of trash; or, 3.b. Provide attached lids on all trash containers that exclude rain, or roof or awning to minimize direct precipitation. 4. Use Efficient Irrigation Systems & Landscape Design The following methods to reduce excessive irrigation runoff shall be considered, and incorporated and implemented where determined applicable and feasible. 4.a. Employing rain shutoff devices to prevent irrigation after precipitation. 4.b. Designing irrigation systems to each landscape area's specific water requirements. 4.c. Using flow reducers or shutoff valves triggered by a pressure drop to control water loss in the event of broken sprinkler heads or l		<i>n</i> e 10	ВМР	YES	NO	N/A
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		4.d.				X
Z. A TITHU AVAMS	5.	Priva	<u> </u>			

		ВМР	YES	NO	N/A
	The d	esign of private roadway drainage shall use at least one of the			
	follow				
	5.a.	Rural swale system: street sheet flows to vegetated swale or			
		gravel shoulder, curbs at street corners, culverts under			X
L		driveways and street crossings.			
	5.b.	Urban curb/swale system: street slopes to curb, periodic swale			X
		inlets drain to vegetated swale/biofilter.			
	5.c.	Dual drainage system: First flush captured in street catch basins			
		and discharged to adjacent vegetated swale or gravel shoulder,			X
l		high flows connect directly to storm water conveyance system.			
	5.d.	Other methods that are comparable and equally effective within			x
		the project.			_^_
6.	Resid	ential Driveways & Guest Parking			
		esign of driveways and private residential parking areas shall use			
		least of the following features.	<u>L</u>		
	6.a.	Design driveways with shared access, flared (single lane at			
		street) or wheelstrips (paving only under tires); or, drain into		×	
		landscaping prior to discharging to the storm water conveyance		^	
<u></u>		system.			
	6.b.	Uncovered temporary or guest parking on private residential lots			
		may be: paved with a permeable surface; or, designed to drain		Х	
		into landscaping prior to discharging to the storm water		_^_	
		conveyance system.			
	6.c.	Other features which are comparable and equally effective.	X		
7.	Dock	Areas			
	Loadi	ng/unloading dock areas shall include the following.			
	7.a.	Cover loading dock areas, or design drainage to preclude urban			x
		run-on and runoff.			_ ^
	7.b.	Direct connections to storm drains from depressed loading			х
İ		docks (truck wells) are prohibited.			
	7.c.	Other features which are comparable and equally effective.			X
8.	Main	tenance Bays			
		tenance bays shall include the following.			
	8.a.	Repair/maintenance bays shall be indoors; or, designed to			Х
		preclude urban run-on and runoff.			^
	8.b.	Design a repair/maintenance bay drainage system to capture all	1		
		wash water, leaks and spills. Connect drains to a sump for			
		collection and disposal. Direct connection of the			X
		repair/maintenance bays to the storm drain system is prohibited.			^
		If required by local jurisdiction, obtain an Industrial Waste			
		Discharge Permit.	1		
	8.c.	Other features which are comparable and equally effective.			Х
9.	+	le Wash Areas			
	 	ty projects that include areas for washing/steam cleaning of			-
	Priori				I
	1	les shall use the following.		İ	
	vehicl	les shall use the following. Self-contained; or covered with a roof or overhang.			Х
	vehicl 9.a.	Self-contained; or covered with a roof or overhang.			X
	vehicl				

		ВМР	YES	NO	N/A	
10.	Outde	oor Processing Areas				
		or process equipment operations, such as rock grinding or				
	crushi	ng, painting or coating, grinding or sanding, degreasing or parts				
		ng, waste piles, and wastewater and solid waste treatment and				
		sal, and other operations determined to be a potential threat to				
		quality by the County shall adhere to the following requirements.	ł			
	10.a.	Cover or enclose areas that would be the most significant source	<u> </u>	-		
		of pollutants; or, slope the area toward a dead-end sump; or,				
		discharge to the sanitary sewer system following appropriate			Х	
		treatment in accordance with conditions established by the				
		applicable sewer agency.				
	10.b.	Grade or berm area to prevent run-on from surrounding areas.	 		Х	
	10.c.	Installation of storm drains in areas of equipment repair is				
	10.0.	prohibited.			X	
	10.d.	Other features which are comparable or equally effective.			X	
11.		oment Wash Areas				
11.		or equipment/accessory washing and steam cleaning activities				
	shall b					
	11.a.				Х	
		Be self-contained; or covered with a roof or overhang.			_^	
	11.b.	Be equipped with a clarifier, grease trap or other pretreatment			Х	
	11 -	facility, as appropriate	1			
	11.c.	Be properly connected to a sanitary sewer.			X	
10	11.d.	Other features which are comparable or equally effective.				
12.		ng Areas				
		ollowing design concepts shall be considered, and incorporated				
		applemented where determined applicable and feasible by the				
	Count	f 				
	12.a.	Where landscaping is proposed in parking areas, incorporate	l x			
		landscape areas into the drainage design.				
	12.b.	Overflow parking (parking stalls provided in excess of the				
		County's minimum parking requirements) may be constructed			Х	
		with permeable paving.				
	12.c.	Other design concepts that are comparable and equally effective.			Х	
13.		ng Area				
	Non-r	etail fuel dispensing areas shall contain the following.				
	13.a.	Overhanging roof structure or canopy. The cover's minimum	1			
		dimensions must be equal to or greater than the area within the				
		grade break. The cover must not drain onto the fuel dispensing				
		area and the downspouts must be routed to prevent drainage			Χ	
		across the fueling area. The fueling area shall drain to the				
		project's treatment control BMP(s) prior to discharging to the				
		storm water conveyance system.				
		Paved with Portland cement concrete (or equivalent smooth				
	13.b.				×	
	13.b.	impervious surface). The use of asphalt concrete shall be			Χ	
	13.b.	impervious surface). The use of asphalt concrete shall be prohibited.			X	
	13.b.	prohibited.			X 	
		•			×	

	ВМР				
13.d.	At a minimum, the concrete fuel dispensing area must extend 6.5 feet (2.0 meters) from the corner of each fuel dispenser, or the length at which the hose and nozzle assembly may be operated plus 1 foot (0.3 meter), whichever is less.			Х	

Please list other project specific Source Control BMPs in the following box. Write N/A if

there are none.		

TREATMENT CONTROL

To select a structural treatment BMP using Treatment Control BMP Selection Matrix (Table 11), each priority project shall compare the list of pollutants for which the downstream receiving waters are impaired (if any), with the pollutants anticipated to be generated by the project (as identified in Table 5). Any pollutants identified by Table 5, which are also causing a Clean Water Act section 303(d) impairment of the receiving waters of the project, shall be considered primary pollutants of concern. Priority projects that are anticipated to generate a primary pollutant of concern shall select a single or combination of stormwater BMPs from Table 11, which maximizes pollutant removal for the particular primary pollutant(s) of concern.

Priority development projects that are <u>not</u> anticipated to generate a pollutant for which the receiving water is CWA 303(d) impaired shall select a single or combination of stormwater BMPs from Table 11, which are effective for pollutant removal of the identified secondary pollutants of concern, consistent with the "maximum extent practicable" standard.

Table 11. Treatment Control BMP Selection Matrix

Pollutants of Concern	Bioretention Facilities (LID)*	Settling Basins (Dry Ponds)	Wet Ponds and Wetlands	Infiltration Facilities or Practices (LID)*	Media Fílters	High-rate biofilters	High-rate media filters	Trash Racks & Hydro -dynamic Devices
Coarse Sediment and Trash	High	High	High	High	High	High	High	High
Pollutants that tend to associate with fine particles during treatment	High	High	High	High	High	Medium	Medium	Low
Pollutants that tend to be dissolved following treatment	Medium	Low	Medium	High	Low	Low	Low	Low

^{*}Additional information is available in the County of San Diego LID Handbook.

NOTES ON POLLUTANTS OF CONCERN:

In Table 12, Pollutants of Concern are grouped as gross pollutants, pollutants that tend to associate with fine particles, and pollutants that remain dissolved.

Table 12

Pollutant	Coarse Sediment and Trash	Pollutants that tend to associate with fine particles during treatment	Pollutants that tend to be dissolved following treatment
Sediment	X	X	
Nutrients		X	X
Heavy Metals		X	
Organic Compounds		X	
Trash & Debris	X		
Oxygen Demanding		X _	
Bacteria		X	
Oil & Grease		X	
Pesticides		X	

A Treatment BMP must address runoff from developed areas. Please provide the post-construction water quality values for the project. Label outfalls on the BMP map. The Water Quality peak rate of discharge flow (Q_{WQ}) and the Water Quality storage volume (V_{WQ}) is dependent on the type of treatment BMP selected for the project.

Outfall	Tributary Area (acres)	QwQ (cfs)	V _{WQ} (ft ³)
	-		

SEE ATTACHED HYDROLOGY REPORT

Please check the box(s) that best describes the Treatment BMP(s) selected for this project.

Biofilters
☑Bioretention swale
☑Vegetated filter strip
Stormwater Planter Box (open-bottomed)
Stormwater Flow-Through Planter (sealed bottom)
☐Bioretention Area
☐Vegetated Roofs/Modules/Walls
Detention Basins
☐Extended/dry detention basin with grass/vegetated
lining
☐Extended/dry detention basin with impervious lining
Infiltration Basins
☐Infiltration basin
■Infiltration trench
☐Dry well
☐Permeable Paving
☐ Gravel
☐ Permeable asphalt
☐Pervious concrete
Unit pavers, ungrouted, set on sand or gravel
☐Subsurface reservoir bed
Wet Ponds or Wetlands
■Wet pond/basin (permanent pool)
☐ Constructed wetland
Filtration
☐Media filtration
☐Sand filtration
Hydrodynamic Separator Systems
Swirl Concentrator
☐ Cyclone Separator
Trash Racks and Screens

Include Treatment Datasheet as Attachment E. The datasheet	COMPLETED	NO
should include the following:		
1. Description of how treatment BMP was designed. Provide a	v	
description for each type of treatment BMP.	^	
2. Engineering calculations for the BMP(s)	X	

Please describe why the selected treatment BMP(s) was selected for this project. For projects utilizing a low performing BMP, please provide a detailed explanation.

BIO-SWALES WERE USED AS A BMP DUE TO THEIR RELATIVE COST EFFICIENCY, EASE OF CONSTRUCTION, AND LOW MAINTENANCE REQUIREMENTS. OFFSITE, SECONDARY BMP IS THE EXISTING GOLF COURSE WHICH IS ALSO A BIO-SWALE.

MAINTENANCE

Please check the box that best describes the maintenance mechanism(s) for this project. Guidelines for each category are located in Chapter 5, Section 5.2 of the County SUSMP.

CATEGORY	SELECTED			
CATEGORY	YES	NO		
First	Х	·		
Second ¹				
Third ¹				
Fourth				

Note:

1. Projects in Category 2 or 3 may choose to establish or be included in a Stormwater Maintenance Assessment District for the long-term maintenance of treatment BMPs.

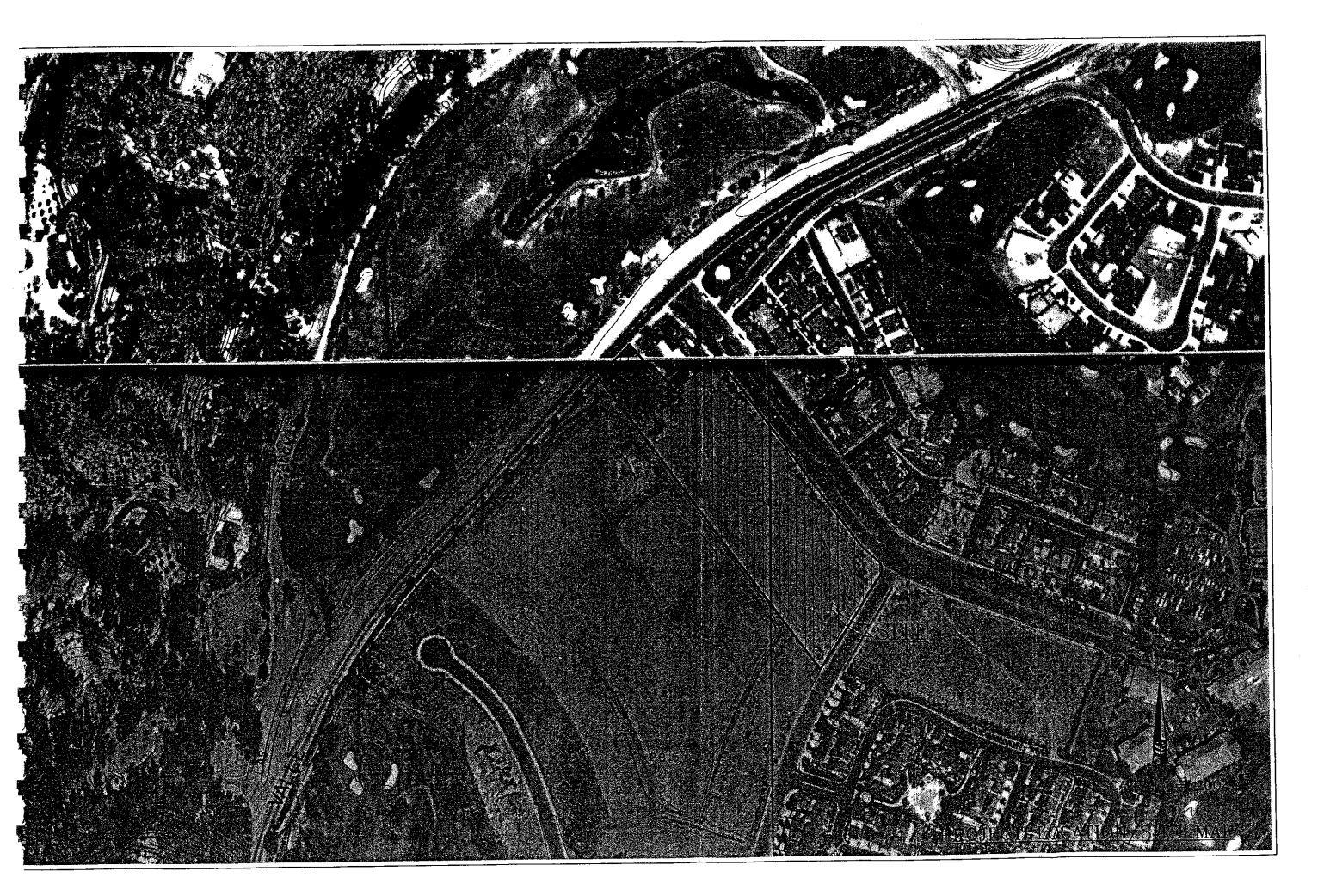
ATTACHMENTS

Please include the following attachments.

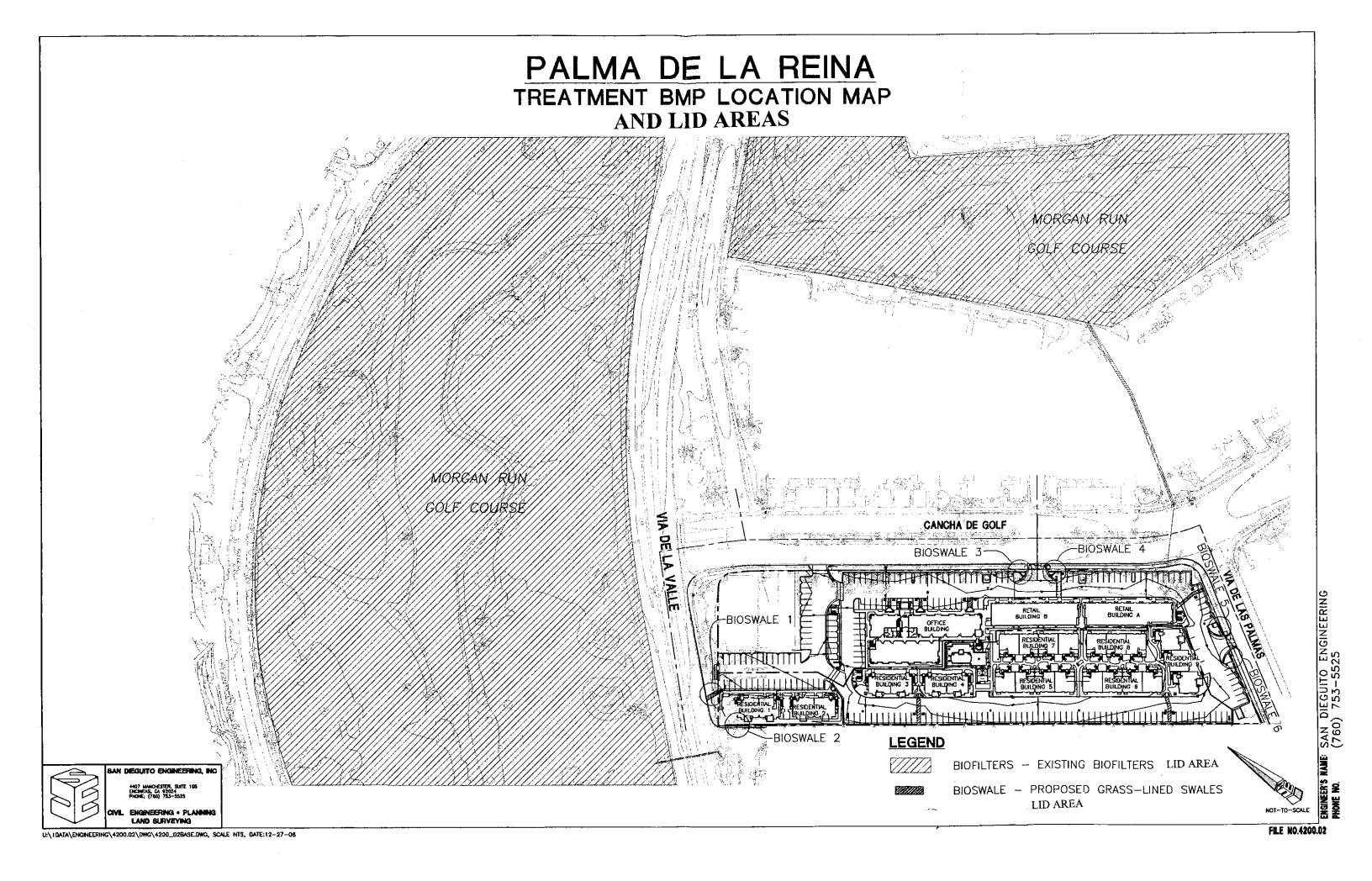
	ATTACHMENT	COMPLETED	N/A
Α	Project Location Map	X	
В	Site Map	X	
С	Relevant Monitoring Data		X
D	LID and Treatment BMP Location Map	X	
E	Treatment BMP Datasheets	X	
F	Operation and Maintenance Program for Treatment BMPs	X	
G	Fiscal Resources	X	
Н	Certification Sheet	X	
I	Addendum	X	

Note: Attachments A and B may be combined.

ATTACHMENTS A&B SITE/LOCATION MAP



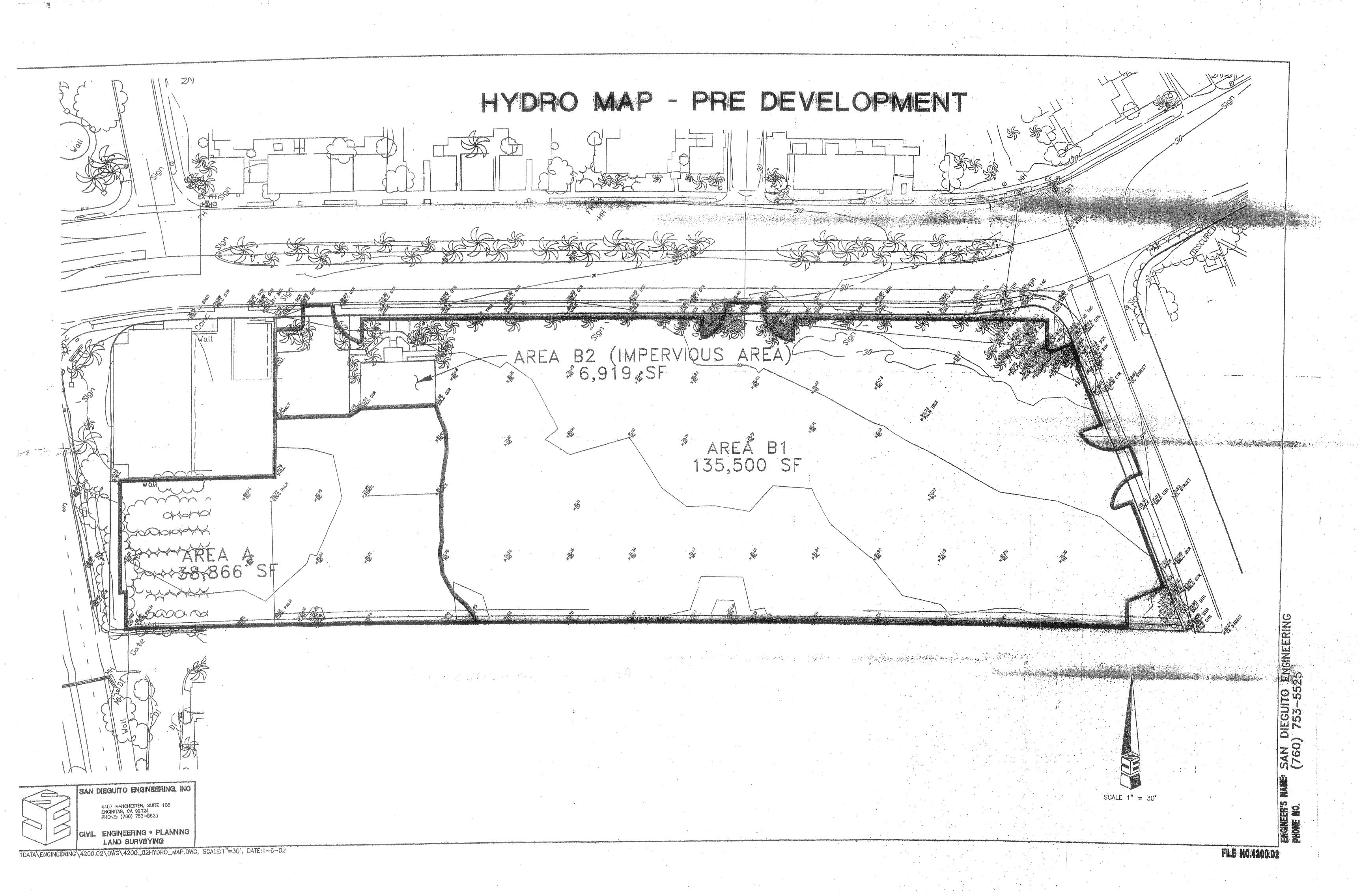
ATTACHMENT D LID AND TREATMENT BMP LOCATION MAP

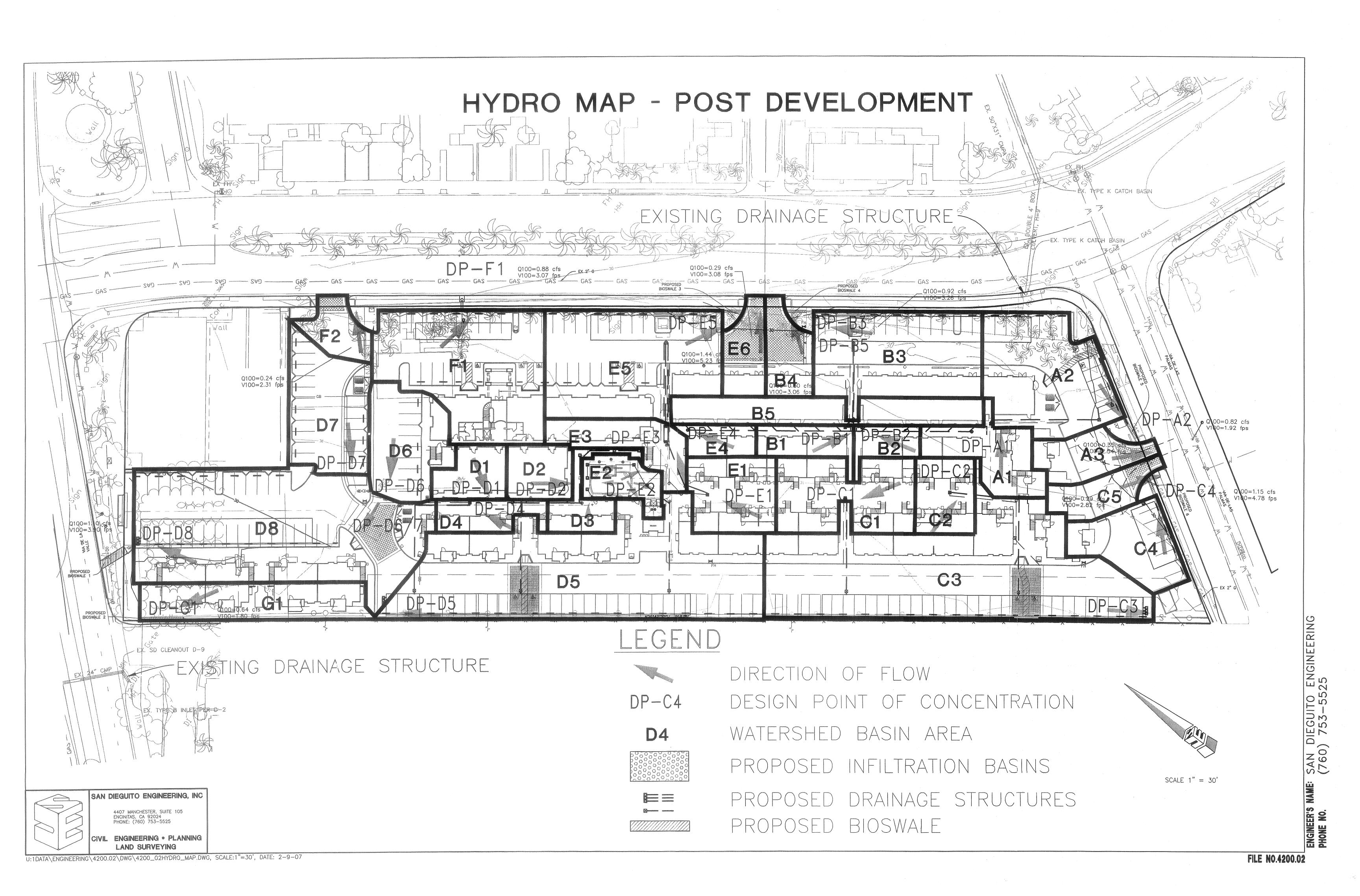


ATTACHMENT E

TREATMENT BMP DATASHEET

(NOTE: POSSIBLE SOURCE FOR DATASHEETS CAN BE FOUND AT WWW.CABMPHANDBOOKS.COM. INCLUDE ENGINEERING CALCULATIONS FOR SIZING THE TREATMENT BMP.)





Bioswale 1 - Q100 Calculations

Friction Method Solve For Normal Depth Part		bloswale 1 -	Q100 Calct	ulations	<u> </u>
Roughness Coefficient 0.030 1/11 1/12 1/1	eron exemples				
Roughness Coefficient	Friction Method	Manning Formula			
Roughness Coefficient	Solve For	Normal Depth		•	
Channel Slope 0.01000 ñ/ft Left Side Slope 3.00 ñ/ft (H:V) Right Side Slope 3.00 ñ/ft (H:V) Bottom Width 4.00 ft Discharge 1.10 ft½s Regular Normal Depth 0.17 ft Flow Area 0.78 ft² Wetted Perimeter 5.09 ft Top Width 5.03 ft Critical Depth 0.13 ft Critical Slope 0.02702 ft/ft Velocity 1.42 ft/s Velocity Head 0.03 ft Specific Energy 0.20 ft Froude Number 0.63 T Flow Type Subcritical Subcritical Subcritical Upstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 0 Upstream Depth 0.00 ft Profile Descri	ggangels.				
Left Side Slope	Roughness Coefficient		0.030		
Right Side Slope 3.00 ft/ft (H:V)	Channel Slope		0.01000 ft	/ft	
Right Side Slope 3.00 ft/ft (H:V)	Left Side Slope		3.00 ft/	/ft (H:V)	
Bottom Width	Right Side Slope				
Normal Depth	Bottom Width				
Normal Depth Flow Area 0.17 ft Flow Area 0.78 ft² Wetted Perimeter 5.09 ft Top Width 5.03 ft Critical Depth 0.13 ft Critical Slope 0.02702 ft/rt Velocity 1.42 ft/s Velocity Head 0.03 ft Specific Energy 0.20 ft Froude Number 0.63 Flow Type Subcritical Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 CVECTION Depth 0.00 ft Profile Description Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Discharge		1.10 ft ^s	/s	
Flow Area 0.78 ft	results.		S. Caus S. Caraca		
Flow Area 0.78 ft Wetted Perimeter 5.09 ft Top Width 5.03 ft Critical Depth 0.13 ft Critical Slope 0.02702 ft/ft Velocity 1.42 ft/s Velocity 1.42 ft/s Velocity Head 0.03 ft Specific Energy 0.20 ft Froude Number 0.63 Flow Type Subcritical Subcritical Subcritical 0.00 ft Number Of Steps 0 One of the subcritical One of	Normal Depth		0.17 ft	- 1 - 1	
Wetted Perimeter 5.09 ft Top Width 5.03 ft Critical Depth 0.13 ft Critical Slope 0.02702 ft/ft Velocity 1.42 ft/s Velocity Head 0.03 ft Specific Energy 0.20 ft Froude Number 0.63 Flow Type Subcritical Downstream Depth O.00 ft Number Of Steps O EXECUTED Force Upstream Depth O.00 ft Profile Description Profile Headloss O.00 ft Infinity ft/s Upstream Velocity Upstream Velocity Infinity ft/s	Flow Area				
Top Width 5.03 ft Critical Depth 0.13 ft Critical Slope 0.02702 ft/ft Velocity 1.42 ft/s Velocity Head 0.03 ft Specific Energy 0.20 ft Froude Number 0.63 Flow Type Subcritical Downstream Depth 0.00 ft Number Of Steps 0 Upstream Depth 0.00 ft Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Wetted Perimeter				
Critical Slope 0.02702 ft/ft Velocity 1.42 ft/s Velocity Head 0.03 ft Specific Energy 0.20 ft Froude Number 0.63 Flow Type Subcritical Subcritical EVERTIFIED 0.00 ft Length 0.00 ft Number Of Steps 0 6 SALE Of Description 0.00 ft Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Top Width		_	•	
Velocity 1.42 ft/s Velocity Head 0.03 ft Specific Energy 0.20 ft Froude Number 0.63 Flow Type Subcritical EVE TOBUTOSIE Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 SVECUTORITIES 0 SVECUTORITIES 0 The Profile Description 0.00 ft Profile Description 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Critical Depth			•	
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Specific Energy 0.20 ft Froude Number 0.63 Flow Type Subcritical CVF Inpti Dat: Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 SVF CutDut Date Upstream Depth 0.00 ft Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Velocity				
Froude Number 0.63 Flow Type Subcritical Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 SVE Outpet Para Upstream Depth 0.00 ft Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Velocity Head		0.03 ft		
Flow Type Subcritical CVE Coption Data Downstream Depth Length Number Of Steps O CVE Cupus Data Upstream Depth Profile Description Profile Headloss Downstream Velocity Upstream Velocity Infinity ft/s	Specific Energy		0.20 ft		
Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 GVE Culiput Para Upstream Depth 0.00 ft Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Froude Number		0.63		
Downstream Depth 0,00 ft Length 0.00 ft Number Of Steps 0 GVE Output Deta Upstream Depth 0.00 ft Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Flow Type	Subcritical			
Length 0.00 ft Number Of Steps 0 CVE ORIGINATE AND THE STEP STEP STEP STEP STEP STEP STEP STE	eventupae e				
Length 0.00 ft Number Of Steps 0 GVE Output Date Upstream Depth 0.00 ft Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Downstream Depth		D 00 #		
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Upstream Depth 0.00 ft Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	•				•
Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	evistomporbejasti				
Profile Description Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	Upstream Depth		0.00 #		
Profile Headloss 0.00 ft Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s	•		0.00 1		
Downstream Velocity Infinity ft/s Upstream Velocity Infinity ft/s			/ 0.00 #		
Upstream Velocity Infinity ft/s	•				
	_				
	-				

0.13

0.01000 ft/ft

0.02702 ft/ft

Critical Depth

Channel Slope

Critical Slope

Cross Section for Bioswale 1 - Q100 Calculations

Man (section light for

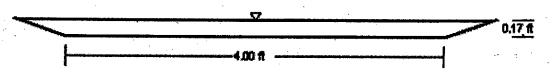
Friction Method

Manning Formula

Solve For

Normal Depth

Roughness Coefficient		0.030	
Channel Slope		0.01000	ft/ft
Normal Depth		0.17	ft
Left Side Slope		3.00	ft/ft (H:V)
Right Side Slope	•	3.00	ft/ft (H:V)
Bottom Width		4.00	ft
Discharge		1.10	ft³/s



Channel Design

Channel Design depth = FSelev - FLelev where FSelev = finished surface clevation (ft) and FL elev = Channel flowline elevation ft.

normal depth = depth of water (H) in channel H1

If Channeldesign depth > normal depth then design ok.

Channel design depth = 28.96' - 28.22' = 0.74' normal depth $= \emptyset.17$

0.74' > 0.17 : ok

Bioswale 1 - Qwq Calculations

Argicalordizaceirleitta							
Friction Method Solve For	Manning Formula Normal Depth		,			-	
याहर्षि वैस्तर्य ।							
Roughness Coefficien Channel Slope Left Side Slope Right Side Slope	•		0.240 0.01000 3.00 3.00	ft/ft ft/ft (H:V) ft/ft (H:V)			
Bottom Width Discharge			4.00 0.15	ft ft ³ /s			
Normal Depth Flow Area			0.18 0.82	ft ft²		22.2	
Wetted Perimeter Top Width Critical Depth			5.14 5.09 0.04	ft ft		. •	
Critical Slope Velocity Velocity Head			2.59279 0.18 0.00	ft/ft ft/s ft	· · .		
Specific Energy Froude Number Flow Type	Subcritical		0.18 0.08	ft			
The state of the s		of many statement	or edesor as kinner y n		eri eranduşe here		
Downstream Depth Length Number Of Steps				ft ft			
Boli dinjugasje			e i de egg				
Upstream Depth Profile Description Profile Headloss				ft			
Downstream Velocity Upstream Velocity Normal Depth		•	Infinity Infinity	ft/s ft/s ft			
Critical Depth Channel Slope Critical Slope			0.04 0.01000	rt R/ft R/ft	• .		

Cross Section for Bioswale 1 - Qwq Calculations

Propert Designation

Friction Method

Manning Formula

Solve For

Normal Depth

ing-11 Unio

Roughness Coefficient	0.240
Channel Slope	0.01000 ft/ft
Normal Depth	0.18 ft
Left Side Slope	3.00 ft/ft (H:V)
Right Side Slope	3.00 ft/ft (H:V)
Bottom Width	4.00 ft
Discharge	 0.15 846

Required Channel Length calculation

LREQ = (Ttreatment min) (Vwa) where

LREQ is the channel length (ff) required to achieve the minimum treatment time of 5 minutes for first flush.

Ttreatment min is the minimum treatment time of Sminutes per San Diego County Standards

Ywa is the channel water velocity (FPS) based on Qwq.

In order to prevent loss of parking spaces, a time of treatment of 2.14 minutes was used, and is to be considered the maximum extent practicable.

$$L = (T_T)(V_{wq})$$

$$T_T = \frac{23.16 \, \text{ft}}{0.18 \, \text{fps}} = 128.4 \, \text{sec} = 2.14 \, \text{min}$$

Bioswale 2 - Q100 Calculations

के देश (दावर किए) कर मुक्ते (कार)			e e e e e e e e e e e e e e e e e e e		
Friction Method	Manning Formula		,	•	
Solve For	Normal Depth				
Que de la composition della co			1987 		San San San San San San San San San San
Roughness Coefficient		0.030			
Channel Slope		0.00500	ft/ft		
Left Side Slope		3.00	ft/ft (H:V)	•	
Right Side Slope		3.00	fl/ft (H:V)	1.	
Discharge		0.64	ft³/s		
State State Commence Commence		\$	Ny sarah		
Normal Depth	v	0.42	ft		••
Flow Area			ft²		
Wetted Perimeter		2.67	ft		•
Top Width		2.53	ft		
Critical Depth		0.31	ft		
Critical Slope		0.02621	ft/ft		
Velocity		1.20	ft/s		
Velocity Head		0.02	ft		r
Specific Energy		0.44	ft ·		
Froude Number		0.46	•		
Flow Type	Subcritical				
	35 424 8 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Server plant scatt Server	m mystice of Agriculture. Subjecting	ne democratic plan refer to the contract	
Downstream Depth		0.00 f	t		
Length		0.00 f	t ·		
Number Of Steps		0		•	
şirin dirilinin					
Upstream Depth		0.00 ft			•
Profile Description					
Profile Headloss	•	0.00 ft	t		-
Downstream Velocity		Infinity ft	/s		
Upstream Velocity		Infinity #	/s	,	
Normal Depth	•	0.42 ft	•		
Critical Depth	- •	0.31 ft			•
Channel Slope	•		/ft		
Critical Slope		0.02621 ft/			
			•	,	

Cross Section for Bioswale 2 - Q100 Calculations

Friction Method

Manning Formula

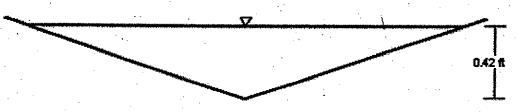
Solve For

Normal Depth

ing the

				•		
Roughness Coefficient		•			0.030	
Channel Slope		ar .		·	0.00500	ft/ft
Normal Depth					0.42	ft
Left Side Slope	.,		* 4.		3.00	ft/ft (H:V)
Right Side Slope	•	,	•		3.00	ft/ft (H:V)
Discharge					0.64	ft³/s

Golden eine Bach



Channel Design

Channel Design depth = FS elev - FL elev where FSelev = finished surface elevation (ft) and FL elev = Channel flowline elevation ft.

normal depth = depth of water (H) in channel H1 based on Q100.

If Channeldesign depth > normal depth then design ok.

Channel Design depth = 30.76'-29.89 = 0.87f+ Normal depth = 0.42ft

0.87ft > 0.42ft .. ok

Bioswale 2 - Qwq Calculations

Resident Sammalin	新 人以多数人的原理。			1001336		
Friction Method	Manning Formula			<i>2</i>		
Solve For	Normal Depth				•	
maggaters; the text						
Roughness Coefficient		0.24	o ·	· · · · · · · · · · · · · · · · · · ·		
Channel Slope		0.0050	O n/n			•
Left Side Slope	·**	3.0	0 ft/ft (H:V)	•		
Right Side Slope		3.0	0 ft/ft (H:V)			
Discharge		0.0	2 ft³/s		•	
The state of the s	to the state of th			11 - 1		**
Normal Depth		0.25	5 ft			
Flow Area		0.19				
Wetted Perimeter		1,59	-			
Top Width		1.50			4.	
Critical Depth		0.08	i fit			
Critical Slope		2.66397	ft/ft			
Velocity		0.11	ft/s		-	•
Velocity Head		0.00	ft			
Specific Energy		0.25	ft			
Froude Number		0.05				
Flow Type	Subcritical					
Gereko idazildek karrikoare	The same way group and the same state of the sam	ducant, communication of the configuration of the	The Art of the Control of the Contro		en and Comment of the comment of the	
Downstream Depth		0.00	ft			
Length		0.00	ft			
Number Of Steps		. 0	•			
Communications			1000			
Upstream Depth	- The state of the	0.00	A	·	<u> </u>	39.59
Profile Description		0.00	. ₹-	•		
Profile Headloss	•	0.00	ft			,
Downstream Velocity		Infinity	ft/s		•	
Upstream Velocity	· ·	Infinity	ft/s			
Normal Depth		0.25	ft .			
Critical Depth		0.08	ft			
Channel Slope		0.00500	ft/ft		•	
Critical Slope		2.66397	ft/ft			
4 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -						

Cross Section for Bioswale 2 - Qwq Calculations

Picelle (2) Lieus Maintine e

Friction Method

Manning Formula

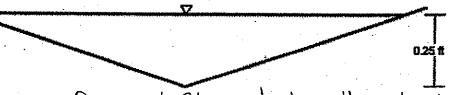
Solve For

Normal Depth

Freist Liefe

Roughness Coefficient			0.240	
Channel Slope	• •	-	0.00500	ft/ft
Normal Depth			0.25	ft
Left Side Slope	•		3.00	ft/ft (H:V)
Right Side Slope	٠.		3.00	ft/ft (H:V)
Discharge			0.02	A3/o

ida Bick Bickeleining beginnter



Required Channel Length calculation

LREQ = (Ttreatment min) (Vwa) where

LREQ is the channel length (A) required to achieve the minimum treatment time of 5 minutes for first flush.

Threatment min is the minimum treatment time of 5 minutes per San Diego County Standards Vwa is the channel water velocity (FPS) based on Qwq.

Vwa = 0.11 fps

Owg CALCULATIONS FOR BIOSWALES 3 - 6

0.09	0.59	C3,C4	0.10		
		. 03.07	00.00	0.80	σ
0	0.20	A2	0.20	0.00	3 (
0.04	0.22	U	3	0.80	זע
	0.33	R3	0.20	0.00	
0.04	0.27	1.0)	000	4
3	75.0	בא	0.20	0.80	
CWQ(CFS)	A (AC)	. 0 00000000000000000000000000000000000	ſ	0 00	2
	= A = (A)	TO RIOSWALE		BIUSWALE" "C" FROM HYDROLOGY MANUAL	DIUSWALE"
	RAINING SUB-AREAS	IN I ENSITY SUB-AREAS DRAINING		- CONTRACTOR OF THE PROPERTY O	

YIELDED A RUN-OFF COEFFICIENT OF 0.80. NOTE: THE RUN-OFF COEFFICIENT USED FOR THESE CALCULATIONS WAS BASED ON TABLE 3-1 OF THE PROJECT SITE IS LOCATED ON SOIL TYPE A. USING THIS DATA IN CONJUCTION WITH TABLE 3-1 SAN DIEGO COUNTY HYDROLOGY MANUAL. ACCORDING TO HYDROLOGIC SOIL GROUPS MAP SHEET 43,

^{*} SEE BMP LOCATION MAP FOR BIOSWALE LOCATIONS.

Bioswale 3 - Qwq Calculations

Friction Method	Manning Formula		:
Solve For	Normal Depth		
Though below the second			
Roughness Coefficient	•	0.240	
Channel Slope	•	0.01000 ft/ft	
Left Side Slope		0.33 ft/ft (H:V)	
Right Side Slope		0.33 ft/ft (H:V)	
Bottom Width		1.00 ft	
Discharge	·	0.04 ft ³ /s	
		1, 160 () (1.00 (1.1 5 () 1.45 () () () 1, 160 () (1.5 () () () () () () () () () ()	
Normal Depth	•	0.21 ft is	
Flow Area		0.22 ft²	
Wetted Perimeter		1.44 ft	
Top Width		1.14 ft	
Critical Depth		0.04 ft	
Critical Slope		2.71544 ft/ft	en en en en en en en en en en en en en e
Velocity		0.18 ft/s	•
Velocity Head	•	0.00 ft	
Specific Energy		0.21 ft	
Froude Number		0.07	·
Flow Type	Subcritical		
eve monthales of a			
Downstream Depth		0.00 ft	
Length		0.00 ft	
Number Of Steps		0	
cvisionnosie program	ersieriskustraktionerisk		
Upstream Depth		0.00 ft	
Profile Description			
Profile Headloss	•	0.00 ft	•
Downstream Velocity	•	Infinity ft/s	
Upstream Velocity		Infinity ft/s	
Normal Depth		0.21 ft	
Critical Depth		0.04 ft	
Channel Slope		0.01000 ft/ft	
Critical Slope		2.71544 ft/ft	

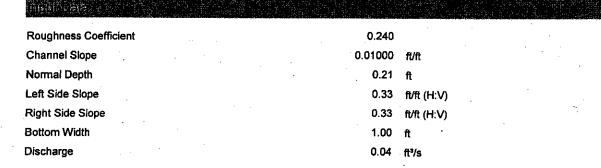
Cross Section for Bioswale 3 - Qwq Calculations

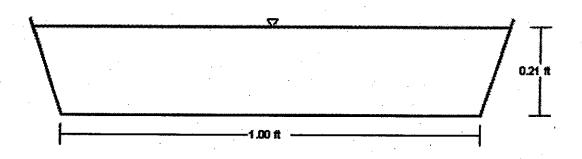
Friction Method

Manning Formula

Solve For

Normal Depth





V:1 📐 H:1

Bioswale 4 - Qwq Calculations

ेर्नाहरस्य विस्तरीति <mark>वर्गा</mark> का ।					
Friction Method	Manning Formula		•		
Solve For	Normal Depth				
and in the second					
Roughness Coefficient	•	0.240)		
Channel Slope		0.01000	ft/ft		
Left Side Slope	•	0.33	ft/ft (H:V)	•	:
Right Side Slope		0.33	ft/ft (H:V)		
Bottom Width	•	1.00	ft		
Discharge		0.04	ft³/s		
	Parametra a aparte de la sella Aparte aparte de la sella de la Sala				
Normal Depth		0.21	ft ·	•	
Flow Area		0.22	ft²		
Wetted Perimeter	•	1.44	ft		
Top Width		1.14	ft .		•
Critical Depth		0.04	ft		
Critical Slope		2.71544	ft/ft		
Velocity		0.18	ft/s		
Velocity Head		0.00	ft		
Specific Energy		0.21	ft		
Froude Number		0.07			
Flow Type	Subcritical				
P.V. Maraka					
Downstream Depth	* * * * * * * * * * * * * * * * * * *	0.00	ft .		
Length	, r	0.00	ft		
Number Of Steps		0			•
eva onfidenti na karaja				Vice of Society. One of the Control of the Control of the Control of the Control of the Control of the Control of the Control o	
Upstream Depth		0.00	ft		
Profile Description					
Profile Headloss	•	0.00	ft		
Downstream Velocity		Infinity	ft/s		
Upstream Velocity		Infinity	ft/s		
Normal Depth		0.21	ft	٠	
Critical Depth	•	0.04	ft		
Channel Slope	•	0.01000	ft/ft		
Critical Slope		2.71544	ft/ft		
**************************************	<u> </u>				

Cross Section for Bioswale 4 - Qwq Calculations

0.04 ft³/s

Eviation	Mathad

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Friction Method

Manning Formula

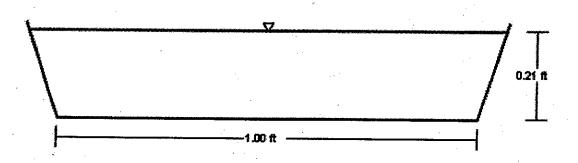
Solve For

Discharge

ម៉េស្សែ (ភ្នំខេ)

Normal Depth

		Month Assemble Street Street
Roughness Coefficient	0.240	
Channel Slope	0.01000	ft/ft
Normal Depth	0.21	ft
Left Side Slope	0.33	ft/ft (H:V)
Right Side Slope	0.33	ft/ft (H:V)
Bottom Width	1.00	ft



v:1 📐 R:1

Bioswale 5 - Qwq Calculations

Majeros Dessalijakaja			
Friction Method	Manning Formula	, where $\varepsilon_{\rm v} = \varepsilon_{\rm v}$	
Solve For	Normal Depth	1	
Tipaki adelka kalanska adele			
Roughness Coefficient		0.240	
Channel Slope		0.01000 ft/ft	
Left Side Slope		0.33 ft/ft (H:V)	
Right Side Slope		0.33 ft/ft (H:V)	
Bottom Width		1.50 ft	
Discharge		0.03 ft³/s	
Elegates			
Normal Depth		0.13 ft	
Flow Area	•	0.13 ft 0.20 ft²	
Wetted Perimeter		1.78 ft	
Top Width		1.59 ft	
Critical Depth		0.02 ft	
Critical Slope		3.04022 ft/ft	
Velocity		0.15 ft/s	
Velocity Head		. 0.00 ft	
Specific Energy	•	0.13 ft	· · · · · · · · · · · · · · · · · · ·
Froude Number		0.07	
Flow Type	Subcritical		•
Downstream Depth		0.00 ft	
Length		0.00 ft	
Number Of Steps		0	
SMCTOMINATION CONTRACT			
Upstream Depth	v.	0.00 ft	
Profile Description			
Profile Headloss		0.00 ft	
Downstream Velocity	,	Infinity ft/s	•
Upstream Velocity		Infinity ft/s	
Normal Depth		0.13 ft	
Critical Depth	•	0.02 ft	
Channel Slope		0.01000 ft/ft	
Critical Slope	·	3.04022 ft/ft	· · · · · · · · · · · · · · · · · · ·

Cross Section for Bioswale 5 - Qwq Calculations

Friction Method

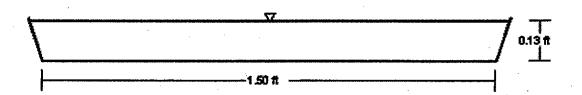
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Manning Formula

Solve For

Normal Depth

Roughness Coefficient	0.240	
Channel Slope	0.01000	ft/ft
Normal Depth	0.13	ft
Left Side Slope	0.33	ft/ft (H:V)
Right Side Slope	. 0.33	ft/ft (H:V)
Bottom Width	1.50	ft
Discharge	0.03	ft³/s



/:1 <u>\</u> H1

Bioswale 6 - Qwq Calculations

Edited President					
Friction Method	Manning Formula		,	,	
Solve For	Normal Depth		•		
विवर्ग ।					
Roughness Coefficient		0.240			
Channel Slope		0.01000	ft/ft		
Left Side Slope		0.33	ft/ft (H:V)		
Right Side Slope		0.33	ft/ft_(H:V)		
Bottom Width	•	1.42	.,		
Discharge		0.09	ft³/s		
Normal Depth		0.27	ft	•	
Flow Area		0.41	ft²		
Wetted Perimeter		2.00	e ft		
Top Width		1.60	ft		
Critical Depth		0.05	ft		
Critical Slope	•	2.44333	ft/ft		4
Velocity		0.22	ft/s	-	
Velocity Head		0.00	ft		• .
Specific Energy		0.28	ft		
Froude Number		0.08	. b≯.		
Flow Type	Subcritical	•	V.		
everbounderen				VIVIOS ELECTRICADA Servicios el VIVIOS E	
Downstream Depth	•	0.00	ft		•
Length		0.00	ft		
Number Of Steps		0			
everimmerke					and the second s
Upstream Depth		0.00	ft		
Profile Description					•
Profile Headloss		0.00	ft		•
Downstream Velocity		Infinity	ft/s		
Upstream Velocity	· ·	Infinity	ft/s		
Normal Depth		0.27	ft .		•
Critical Depth		0.05	ft		
Channel Slope		0.01000	ft/ft		•
Critical Slope		2.44333	ft/ft		
					· · · · · · · · · · · · · · · · · · ·

Cross Section for Bioswale 6 - Qwq Calculations

Friction	Markad
LUCRON	MELITOD

Manning Formula

Solve For

Normal Depth

Roughness Coefficient

Proffes Carrolling

0.240

Channel Slope

0.01000 ft/ft

Normal Depth

0.27 ft

Left Side Slope

0.33 ft/ft (H:V)

Right Side Slope

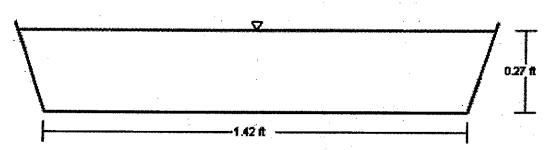
0.33 ft/ft (H:V)

Bottom Width

1.42 ft

Discharge

0.09 ft³/s



at the bank lining interface as well as maintaining a stable natural channel. The bank lining shall extend below calculated scour depths at the lining interface, and provide the minimum freeboard as outlined in Section 5.3.7.



Figure 5-5 Example of Bank-Lined Channel

5.4.3 Bio-Engineered Channel Stabilization

Traditionally, the "hard lined" channel stabilization techniques (i.e., riprap, gabion, concrete, etc.) have been used to stabilize erosion problem areas. Bioengineering is an applied science that integrates structural, biological and ecological principles to construct living structures (plant communities) for erosion, sediment, and flood control purposes. In many instances, "bioengineered" channel stabilization measures can be safely utilized in place of "hard lined" measures, and these methods are encouraged whenever practical. Successful application of "bioengineered" stabilization measures depends upon accurate diagnosis of the causes of channel stability problems, rather than just treating visible problem areas. Section 5.13 provides useful resources on bioengineered solutions for natural channel stabilization.

5.5 DESIGN CRITERIA – GRASS-LINED CHANNELS

This Section presents minimum design criteria for grass-lined channels. The design engineer is responsible for confirming that a channel design meets these criteria, the general open-channel criteria outlined in Section 5.3, and any special considerations for a particular design situation.

5.5.1 Longitudinal Channel Slopes

Grass-lined channel slopes are dictated by maximum permissible velocity requirements. Where the natural topography is steeper then desirable, drop structures may be utilized to maintain design velocities. Grass-lined channels shall have a minimum longitudinal gradient of 0.5 percent whenever practical (see Section 5.3.4).

5.5.2 Roughness Coefficient

Appendix A (Table A-4) provides appropriate Manning roughness coefficients for grass-lined channels. The Manning roughness coefficient used in evaluating channel capacity shall assume a mature channel (i.e., substantial vegetation with minimal maintenance). For evaluating channel slope and permissible velocity, the Manning roughness coefficient shall assume a freshly mowed condition.

Rock-Lined Trickle Channel

Rock-lined trickle channels shall have a minimum depth of 12 inches, with the Manning roughness coefficient determined as described in Section 5.7.17. The minimum stone size for rock-lined trickle channels shall be 6 inches.

5.5.3.2 Low Flow Channels

Low-flow channels are used to contain relatively frequent flows within a recognizable channel section. Low-flow channels are recommended for channels with a 100-year flow greater than 200 cfs, and at a minimum have the capacity to convey the 2-year flow event with no freeboard. The overall flow capacity of the channel shall include the capacity of the low flow channel.

Low-flow channels shall have a minimum depth of 12 inches. The side slopes of the low-flow channel shall be 2.5H:1V to 3H:1V whenever practicable. The main channel depth limitations (Section 5.5.5) do not apply to the low-flow channel area of the overall channel cross-section.

5.5.4 Bottom Width

The selection of the overall channel bottom width shall consider factors such as possible wetland mitigation requirements, constructability, channel stability and maintenance, multi-use purpose, and width of the low flow channel (if any). 1.0 Ft For Hous mixthen

5.5.5 Freeboard and Flow Depth

Swales and grass-lined channels conveying a 100-year flow less than or equal to 10 cfs shall have a minimum freeboard of 6 inches. Grass-lined channels conveying larger discharges shall meet the minimum freeboard requirements outlined in Section 5.3.7.

The recommended design depth of flow for a grass-lined channel (outside the low flow channel arca) is 5.0 feet for a 100-year flow of 1,500 cfs or less whenever practical. Excessive depths shall also be avoided in channels with greater design flows to the maximum extent practicable. Section 5.3.9 discusses access and safety for open channels, including thresholds for flow depth and velocity.

5.5.6 Side Slopes

Side slopes of a grass-lined channel shall be not be steeper than 3H:1V.

5.5.7 **Grass Lining**

Satisfactory performance of a grass-lined channel depends on constructing the channel with the proper shape and preparing the area in a manner to provide conditions favorable to vegetative growth. Between the time of seeding and the actual establishment of the grass, the channel is unprotected and subject to considerable damage unless interim erosion protection is provided. Jute, plastic, paper mesh, hay mulch may be used to protect the waterway until the vegetation becomes established.

The grass lining for channels may be seeded or sodded with a grass species that is adapted to the local climate and will flourish with minimal irrigation. Channel vegetation is usually established by seeding. In the more critical sections of some channels, it may be desirable to provide immediate protection by transplanting a complete sod cover. All seeding, planting, and sodding shall conform to local landscape recommendations.

5.5.8 Horizontal Channel Alignment and Bend Protection

The potential for erosion increases along the outside bank of a channel bend due to the acceleration of flow velocities on the outside part of the bend. Thus, it is often necessary to provide erosion protection in natural or grass-lined channels that otherwise would not need protection.

The minimum radius for channels with a 100-year runoff of 20 cfs or less shall be 25 feet. For channels carrying larger flows, horizontal channels alignment shall be limited based on the presence of erosion protection.

No channel bend protection is required along bends where the radius is greater than two times the top width of the 100-year water surface or the channel is constructed in erosion-resistant soils. Channels without bend protection are not allowed to have a curvature with a radius of less than two times the 100-year flow top width or less than 100 feet, whichever is greater.

Channels bends built in areas with erosive soil conditions shall always have erosion protection. When erosion protection is provided, channels are allowed to have minimum radius equivalent to 1.2 times the 100-year flow top width, but in no case shall the radius of curvature be less than 50 feet.

Erosion protection shall extend downstream from the end of the bend a distance that is equal to the length of the bend measured along the channel centerline.

5.5.9 Maintenance

Grass-lined channels shall be maintained to ensure that vegetation is removed or maintained on a regular basis to maintain the function of the facility. The project owner shall ensure that appropriate mechanism is in place to provide maintenance for the lifetime of the facility.

5.6 DESIGN CRITERIA – WETLAND BOTTOM CHANNEL

This Section presents minimum design criteria for wetland-bottom channels. The design engineer is responsible for confirming that a channel design meets these criteria, the general open-channel criteria outlined in Section 5.3, and any special considerations for a particular design situation.

When designing a wetland-bottom channel, the design engineer must consider both the interim ("new channel") condition and ultimate ("mature channel") condition. For the interim condition, the channel shall maintain non-erosive velocities under the design flow (Section 5.6.1). The design engineer shall evaluate the channel conveyance capacity under ultimate conditions (Section 5.6.2).

5.6.1 Longitudinal Channel Slope

The design engineer shall establish a longitudinal channel slope that maintains non-erosive velocities during the interim condition (a.k.a. the "new channel" condition), assuming minimal or immature wetland vegetation in the channel bottom. Table 5-13 (page 5-43) provides guidelines for maximum permissible velocity. Wetland- bottom channels shall maintain a minimum longitudinal slope of 0.5 percent whenever practicable (see Section 5.3.4).

The design engineer may increase the maximum permissible velocity when temporary erosion control measures are properly installed and maintained during the interim condition. The design engineer may also employ temporary grade control structures to reduce the effective slope of the channel during the interim condition. The Froude Number for wetland-bottom portions of a channel during the interim condition shall not exceed $F_R = 0.7$. Where topography is steeper than desirable, permanent drop structures may be used to maintain design velocities.

ATTACHMENT F

OPERATION AND MAINTENANCE PROGRAM FOR TREATMENT BMPS

(NOTE: INFORMATION REGARDING OPERATION AND MAINTENANCE CAN BE OBTAINED FROM THE FOLLOWING WEB SITE:

HTTP://WWW.CO.SAN-DIEGO.CA.US/DPW/WATERSHEDS/LAND_DEV/SUSMP.HTML.)

CONTRACTOR NAME	CONTACT PERSON	PHONE NUMBER	JOB DESCRIPTION	STATUS
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SITE INSPECTION RECORD

DATE OF INSPECT N	DATE OF STORM	AMOUNT OF PRECIPITATION (INCHES)	OBSERVATION	FOLLOW UP WORK PERFORMED (IF NECESSARY)	NAME OF INSPECTOR
					
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NATIONAL WEATHER SERVICE RECORDING (858) 289-1212

STORM WATER POLLUTION PREVENTION PLAN (SWPPP) BEST MANAGEMENT PRACTICES (BMP)

FOR L-14372

RESPONSIBLE PERSONNEL: RICHARD CAVANALIGH, PHONE NO. (858) 756-3771

INSPECTING EQUIPMENT: ALL CONTRACTORS AND SUBCONTRACTORS

INSPECTING OF BMP'S: RICHARD CAVANALISH

TRAINING EMPLOYEES ABOUT BMP'S AFFECTING THEIR JOB: ALL CONTRACTORS AND SUBCONTRACTORS

SOURCE IDENTIFICATION:

THE PRIMARY POTENTIAL SQURCE OF POLLUTANTS IS SOIL DISTURBED BY CONSTRUCTION OPERATIONS. IF NOT CONTROLLED, SOIL MAY BE CARRED OFF—STE IN THE FORM OF SEDMENT BY STORM WATER DISCHARCE. OTHER POTENTIAL SQURCES OF POLLUTANTS INCLUDE FUEL SPILLS AND LEAKS FROM THE DIFFERENT VEHICLES AND HEAVY EARTH MOVING EQUIPMENT USED DURING CONSTRUCTION OPERATIONS. ADDITIONALLY, SOLVENTS, OIL BASED PAINTS, AND DTHER MISCELLANEOUS HYDROCARBONS USED DURING CONSTRUCTION OPERATIONS ARE POTENTIAL SOURCES OF POLLUTANTS.

SITE PLAN GENERAL DISCUSSION AND ASSOCIATED INFORMATION:

THE GRADING PLAN CONSISTS OF SHEETS 1-8. THESE SHEETS ARE COMPILED FROM PLANS DEVELOPED AND PROCESSED THROUGH THE DOVERNING AGENCY. THEY MAY INCLIDE INFORMATION FROM MASS GRADING PLANS, ROUGH GRADING PLANS, ROUGH CHANS, AND LANDSCAPING PLANS WHICH PERTAIN TO BEST MANAGEMENT PRACTICES PREVENTION. THEY COMPRISE THE BEST MANAGEMENT PRACTICES PLAN FOR CONTROL OF SEDMENT AND OTHER POLLUTANTS DURING CONSTRUCTION OF THE PROJECT. SINCE CONSTRUCTION PROJECTS ARE OTHANIC AND CHANGE FROM DAY TO DAY, SITE CONDITIONS MAY DIFFER FROM THOSE SHOWN ON THE SITE PLAN,

THE ULTIMATE GOAL AT THE END OF CONSTRUCTION IS THAT ALL SLOPES WILL BE PROTECTED BY MAN MADE OR NATURAL BMP'S AND THE TRANSFER OF SEDIMENT AND OTHER POLLUTANTS FROM THE SITE WILL NOT BE GREATER THAN THAT PRIOR TO DEVELOPMENT.

- LOCATION OF CONTROL PRACTICES USED DURING CONSTRUCTION:
- LOCATION OF CONTROL PRACTICES USED DURING CONSTRUCTION;
 AREAS OF CUT AND FILL;
 DRAINAGE PATTERNS AND SLOPES ANTICIPATED AFTER MAJOR
 GRADING ACTIVITIES ARE COMPLETED;
 AREAS OF SOIL DISTURBANCE:
 SURFACE WATER LOCATIONS;
 AREAS OF POTENTIAL SOIL EROSION WHERE CONTROL PRACTICES
 WILL BE USED DURING CONSTRUCTION;
 EDISTING AND PLANNED PAVED AREAS AND BUILDINGS (IF KNOWN);
 VEHICLE STORAGE AND SERVICE AREAS; TO BE SHOWN BY CONTRACTOR
 AREAS OF EXISTING VEGETATION TO BE RETAINED.

REQUIRED DISCUSSION:

Tokic materials that are known to have been treated, stored, disposed, spilled, or leaked in significant quantities onto the construction site:

NO SIGNIFICANT QUANTITIES OF TOXIC MATERIALS HAVE BEEN TREATED, STORED, DISPOSED, SPILLED OR LEAVED ON THE CONSTRUCTION SITE. DISECT, FUEL FOR CONSTRUCTION OPERATIONS HAS BEEN TEMPORARRY STORED DNI-SITE IN ABOVE ORDUND TANKS. MINOR SPILLING OF DIESEL FUEL OR MOTOR DIL MAY OCCUR DURING FLIELING AND MANTEMANCE OPERATIONS. SUCH MINOR SPILLS SHALL BE BROUGHT TO THE ATTENTION OF THE CONSTRUCTION SUPERINTEDENT WHO SHALL ARRANGE FOR PROPER COLLECTION AND DISPOSAL.

METHODS OF ON-SITE STORAGE AND DISPOSAL OF CONSTRUCTION MATERIALS:

POLLUTANTS (EXCLUDING SEDIMENT) ARE DISPOSED OF OFF-SITE AT DESIGNATED L'ANDFILLS, USING PROPER HANDLING TECHNIQUES, AND ACCOMPANIED BY REQUIRED MANIFESTATION.

THE NATURE OF FILL MATERIAL AND EXISTING DATA DESCRIBING THE SOIL ON THE CONSTRUCTION SITE:

COPIES OF SOILS INVESTIGATIVE REPORTS CAN BE OBTAINED WITH THE OWNER'S PERMISSION FROM THE SOILS ENGINEER LISTED ON THE GRADING PLAN.

POLLUTANTS (OTHER THAN SEDIMENT) THAT ARE LIKELY TO BE PRESENT IN STORM WATER DISCHARGES IN SIGNIFICANT QUANTITIES:

THERE ARE NO POLLUTANTS OTHER THAN SEDIMENT THAT ARE LIKELY TO BE PRESENT IN STORM WATER DISCHARGES IN SIGNIFICANT

CONSTRUCTION SITE AREAS

RUN-OFF COEFFICIENT PRIOR TO CONSTRUCTION: 0.30

RUN-OFF COEFFICIENT AFTER CONSTRUCTION: 0.80

IN DE AREA IMPERVIOUS PRIOR TO CONSTRUCTION: ON

X OF AREA IMPERVIOUS AFTER CONSTRUCTION: 96.5X

PRACTICES TO MINIMAZE CONTACT OF CONSTRUCTION MATERIALS, EQUIPMENT AND VEHICLES WITH STORM WATER:

FUELING, CLEANING AND MAINTENANCE SHOULD BE PERFORMED AT LOCATIONS THAT ARE FLAT, AND GEOGRAPHICALLY CONTAINED IN ORDER TO FACRITATE CLEANING UP ANY ASSOCIATED SPILLS. ALL TEMPORRY FUEL TANKS LOCATED ON-SITE SHALL BE CONTAINED WITHIN EARTHEN BERMS LINED BY MSQUEEN. VEHICLES AND EQUIPMENT SPREADS MAY BE MOVED FROM TIME TO TIME TO VARIOUS LOCATIONS WITHIN THE PROJECT FOR FUELING, CLEANING, AND MAINTENANCE.

CONSTRUCTION MATERIAL LOADING, UNLOADING AND ACCESS AREAS:

MATERIAL LOADING, UNLOADING AND ACCESS AREAS MAY DOCUR ANYWHERE ON-SITE THAT IS FLAT, CONTAINED BY GEOGRAPHY, AND IS NOT EXPECTED TO BE INUNDATED WITH WATER DURING RAIN STORMS.

PRECONSTRUCTION CONTROL PRACTICES (IF ANY) TO REDUCE SEDIMENT AND OTHER POLLUTANTS IN STORM WATER DISCHARGES;

DISCUSSIONS SHALL OCCUR BETWEEN SITE CONSTRUCTION MANAGEMENT PERSONNEL AND CONTRACTORS PRIOR TO CONSTRUCTION OF ANY GIVEN PHASE OF WORK (i.e. STORM DRAIN CONSTRUCTION). AT THOSE PRE—CON MEETINGS BUP AND IDEAS FOR OTHER CONSTRUCTION POLLUTION PREVENTION MEASURES WILL BE DISCUSSED.

AS MENTIONED ABOVE, THE PRIMARY STORAGE, CLEANING AND MAINTENANCE AREAS WILL BE VARIOUS AREAS ON-SITE THAT ARE SECURED BY GEOGRAPHY AND NOT INJUNDATED DURING RAINSTORMS.

BEST MANAGEMENT PRACTICES (8MP) FOR EROSION CONTROL AND SEDIMENT CONTROL.

THE FOLLOWING BEST MANAGEMENT PRACTICES FOR EROSION AND SEDIMENT CONTROL ARE SHOWN ON THE ATTACHED PLANS. SOME ARE FOR SHORT TERM MITIGATION DURING THE CONSTRUCTION PROCESS AND OTHERS ARE FOR LONG TERM MITIGATION AFTER CONSTRUCTION IS COMPLETED.

SITE DEVELOPMENT MANAGEMENT
SHALL ENSURE THAT ALL PERSONNEL INVOLVED IN INSPECTIONS,
MAINTENANCE, AND REPAIR OF BMP'S, UNDERSTAND AND ARE QUALIFIED TO
PERFORM SUCH TASKS. MEETINGS SHALL BE HELD PERIODICALLY BETWEEN
THE VARIOUS CONTRACTORS AND THE ON-STRUCTION
SUPERINTENDENT TO DISCUSS POTENTIAL PROBLEMS OR EXISTING PRACTICES
THAT CAN BE IMPROVED IN ORDER TO PREVENT POLLUTION DURING FUELING,
CLEANING OR MAINTENANCE OPERATIONS.

EROSION AND SEDIMENT CONTROL:

LISTED BELOW ARE BEST MANAGEMENT PRACTICES TO BE USED FOR SOIL STABILIZATION PURPOSES. THEY ARE SHOWN IN DETAIL ON THE FOLLOWING SHEETS. THESE BUP'S HAVE BEEN SELECTED FROM PROVEN METHODS USED FOR CONTROLLING EROSION WITHIN THE GENERAL PROJECT AREA.

SHORT TERM - CONSTRUCTION BMP'S:

- EARTHEN BERMS
 GRAVEL BAGS
 SILT FENCES
 TEMPORARY STORM DRAINS
- DESILTING BASINS
 NETTING AND MATTING
 CABION MATS AND CUT-OFF WALLS
 RIP RAP
- ENERGY DISSIPATORS
 BLADED SWALES

CONTROL PRACTICES TO REDUCE THE TRACKING OF SEDIMENT ONTO PUBLIC AND PRIVATE ROADS:

PRIVATE ROADS:
TRACKING OF SECIMENT ONTO PUBLIC AND PRIVATE ROADS SHALL BE
MITIGATED BY THE INSTALLATION OF GRAVEL PADS AT POINTS OF INGRESS AND EGRESS.

CONTROL PRACTICES TO REDUCE WIND EROSION:

GRADING OPERATIONS WILL COMPLY WITH COUNTY ORDINANCES RELATING TO DUST CONTROL. ADDITIONALLY, ALL FILL MATERIAL WILL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY AND LANOSCAPED OR HYDROSEEDED AS REQUIRED.

NON STORM WATER MANAGEMENT:

THE DNLY NON STORM WATER DISCHARGES ANTICIPATED TO DRAIN OFF-SITE WILL BE FROM LANDSCAPE IRRIGATION, PIPE FLUSHING AND TESTING, STREET WASHING AND POSSIBLE DEWATERING OPERATIONS.

RECOGNIZED AND ALLOWED BY THE GENERAL PERMIT.

POST CONSTRUCTION STORM WATER MANAGEMENT:

PERMANENT BMP'S TO NITICATE POLLUTANTS AFTER ALL CONSTRUCTION PHASES HAVE BEEN COMPLETED AT THE SITE FOLLOW. UNITE, SUCH BMP'S ARE ACCEPTED BY THEY WILL BE MANTANED BY THE OWNER. SUBSEQUENT ENTITIES,

LONG TERM - POST-CONSTRUCTION BMP'S:

- PERMANENT VEGETATION AND IRRICATION PRACTICES ENERGY DISSIPATIORS CUT-OFF WALLS RP RAP WALLS AND HEAD-WALLS PERMANENT STORM DRAINS AND ATTACHED STRUCTURES

SITE MAINTENANCE, INSPECTION AND REPAIR:

INSPECTIONS OF THE CONSTRUCTION SITE SHALL BE CONDUCTED PRIOR TO ANTICIPATED STORM EVENTS AND AFTER ACTUAL STORM EVENTS TO IDENTIFY AREAS CONTRIBUTIONS OF A STORM WATER DISCHARGE ASSOCIATED WITH CONSTRUCTION ACTURY AND TO EVALUATE WHETHER MESSURES TO REDUCE POLLUTIANT LOADINGS IDENTIFIED ON THIS PLAN ARE ADEQUATE AND PROPERTY IMPLEMENTED IN ACCORDANCE WITH THE TERRIS OF THE PERMIT OR WHETHER ADDITIONAL CONTROL MEASURES ARE NEEDED. A RECORD OF THE INSPECTIONS MIST INCLUDE THE DATE OF THE INSPECTION, THE NOMBULAL(S) WHO PERFORMED THE INSPECTIONS WILL DECIDE THE APPROPRIATIONS. THE PERSONS PERFORMING THE SITE INSPECTIONS WILL DECIDE THE APPROPRIATIONS OF INFLEMENTING THE BUP'S SHOWN ON THIS PLAN AT ANY PARTICULAR CONSTRUCTION PHASE. ADDITIONALLY, THEY WILL DETERMINE IN THE APPROPRIATE BUP'S AFE WORKING, OR NEED TO BE REPARED, ENHANCED OR REPLACED.

MONITORING RECORDS:

RECORDS OF ALL INSPECTIONS, COMPLIANCE CERTIFICATIONS, AND MONCOMPLIANCE REPORTING MUST BE RETAINED FOR A PERIOD OF AT LEAST THREE YEARS. WITH THE EXCEPTION OF NONCOMPLIANCE REPORTING, DISCHARGES ARE NOT REQUIRED TO SUBMIT THESE RECORDS.

WATER CONSERVATION:

MAINTAIN WATER DISTRIBUTION EQUIPMENT

REPAIR LEAKS PROMPTLY

SWEEP PAYED AREAS

- III. COLLECT DEBRIS PROMPTLY IV. AND DISPOSE OF PROPERLY

STORM WATER PROTECTION NOTES:

- 1. DURING THE RAINY SEASON THE AMOUNT OF EXPOSED SOIL ALLOWED AT ONE THE SHALL NOT EXCEED THAT WHICH CAN BE ADEQUATELY PROTECTED BY THE PROPERTY OWNER IN THE EVENT OF A MAINSTORM 125% OF ALL SUPPLIES MEDIED FOR THAP MEASURES SHALL BE RETAINED ON THE JOB STE IN A MANNER THAT ALLOWS FULL. REPLOYMENT AND COMPLETE INSTALLATION IN 48 HOURS OR LESS OF A FORECAST RAIN.
- 2. NO AREA BEING DISTURBED SHALL EXCEED 50 ACRES AT ANY ONEN TIME WITHOUT DEMONSTRATING TO THE SAN DIECO COUNTY PRIV DIRECTOR'S SANS'ACTION THAT AGOUNT EXISION AND SEDMENT CONTROL ON BE MANUFAMED. ANY DISTURBED ANEA THAT IS NOT ACTIVILY CRADED FOR 10 DAYS MAST BE FULLY PROTECTED FROM EROSON, URBL ADEQUARE LONG-TERM PROTECTIONS ARE INSTALLED. THE DISTURBED AREA SHALL BE INCLUDED WHEN CALCULATING THE ACTIVE DISTURBANCE AREA. ALL EROSON CONTROL MEASURES SHALL REMAIN INSTALLED AND MAINTAINED DURING
- 3. THE PROPERTY OWNER IS OBLIGATED TO DISURE COMPLIANCE WITH ALL APPLICABLE STORMMATER REGULATIONS AT ALL TIMES. THE BUP'S (BEST MANAGEMENT PRACTICES)
 THAT HAVE BEEN INCORPORATED INTO THIS PLAN SHALL BE IMPLEMENTED AND INVIT MAKE MEZER MALURGENNIED MILD THIS PLAN SHALL BE AMPLIBEMEND AND MAMITIAMED TO EFFECTMENT PREVENT THE POTENTIALLY NEGATIVE IMPACTS OF THIS PROJECT'S CONSTRUCTION ACTIVITIES ON STORMANIES COLLITY. THE MAMPISMANCE OF THE BMY'S IS THE PERMITE'S RESPONSIBILITY, AND FAILURE TO PROPERTY INSTALL OR MANURUM THE BMY'S MAY RESLAT IN EMPORCEDIENT ACTION BY THE COUNTY OF SAN DEGO OR OTHERS. IF INSTALLED BMY'S FAIL, THEY MAST BE REPLACED WITH AM ACCEPTABLE ALTERNATE MITHIN 24 HOURS, OR AS SOON AS SAFE TO DO SO.

BMP MAINTENANCE OBLIGATIONS:

1. The permitee of the grading shorin on these plans acknowledges that bap's (best management practices) have been incorporated into this plan and will be implemented and maintained to effectively minimize the potentially NEGATINE MEPACIS OF THIS PROJECT'S CONSTRUCTION ACTIVITIES ON STORMMATER COULTY, FURTHER, THAT THE MAINTENANCE OF THE SELECTED STORMMATER BUP'S SHOWN ON THIS PLAN IS THEIR RESPONSIBILITY, AND FAILURE TO PROPERTY INSTALL OR MANITAIN THE BAR'S MAY RESULT IN ENFORCEMENT ACTION BY THE COUNTY OF SAN DIEGO OR OTHERS.

ATTACHMENT G FISCAL RESOURCES

ATTACHMENT C-Mechanisms to Assure Maintenance

FIRST CATEGORY:

The County should have only minimal concern for ongoing maintenance. The proposed BMPs inherently "take care of themselves", or property owners can naturally be expected to do so as an incident of taking care of their property

Typical BMPs:

- ☐ Biofilters (Grass swale, Grass strip, vegetated buffer)
- :: Infiltration BMP (basin, trench);

For Palma De La Reina by Newport Pacific, First Category BMP's are: Grass swales as shown on the BMP map that is a part of this SWMP. These BMP's will be maintained by the property owner as a part of their regular property maintenance. Annual maintenance is estimated at \$1,250 per Appendix H of the SUSMP Manual.

Funding:

None Required.

Mechanisms to Assure Maintenance

See notes 1 through 5 on following page.

SECOND CATEGORY:

The County needs to assure ongoing maintenance. The nature of the proposed BMPs indicates that it is appropriate for property owners to be given primary responsibility for maintenance; on a perpetual basis (unless a storm water utility is eventually formed). However, the County (in a "backup" role) needs to be able to step in and perform the maintenance if property owner fails, and needs to have security to provide funding for such backup maintenance. Security for "backup" maintenance after the interim period (5 years) would not be provided, however primary owner maintenance responsibility would remain. If a storm water utility or other permanent mechanism is put into place, it could assume either a primary or backup maintenance role.

Typical BMPs:

- · Biofilters;
- · Small Detention Basins;
- Infiltration BMP, and:
- Single Storm Drain Inserts, Oil/Water separator, Catch basin insert & screens.

For Palma De La Reina by Newport Pacific, Second Category BMP's are: Biofiltration through the existing golf course.

Funding:

No maintenance funding is required for the second category BMP.

Mechanisms to Assure Maintenance:

- 1. Storm water Ordinance Requirement: The WPO requires this ongoing maintenance. In the event that the mechanisms below prove ineffective, or in addition to enforcing those mechanisms, civil action, criminal action or administrative citation could also be pursued for violations of the ordinance.
- 2. Public Nuisance Abatement: Under the WPO failure to maintain a BMP would constitute a public nuisance, which may be abated under the Uniform Public Nuisance Abatement Procedure. This provides an enforcement mechanism additional to the above, and would allow costs of maintenance to be billed to the owner, a lien placed on the property, and the tax collection process to be used.
- 3. Notice to Purchasers. Section 67.819(e) of the WPO requires developers to provide clear written notification to persons acquiring land upon which a BMP is located, or others assuming a BMP maintenance obligation, of the maintenance duty.
- 4. Conditions in Ongoing Land Use Permits: For those applications (listed in WPO Section 67.804) upon whose approval ongoing conditions may be imposed, a condition will be added which requires the owner of the land upon which the storm water facility is located to maintain that facility in accordance with the requirements specified in the SMP. Failure to perform maintenance may then be addressed as a violation of the permit, under the ordinance governing that permit process.
- 5. Subdivision Public Report: Tentative Map and Tentative Parcel Map approvals will be conditioned to require that, prior to approval of a Final or Parcel Map, the subdivider shall provide evidence to the Director of Public Works, that the subdivider has requested the California Department of Real Estate to include in the public report to be issued for the sales of lots within the subdivision, a notification regarding the maintenance requirement. (The requirement for this condition would not be applicable to subdivisions which are exempt from regulation under the Subdivided Lands Act, or for which no public report will be issued.)
- 6. BMP Maintenance Agreement with Easement and Covenant: An agreement will be entered into with the County, which will function three ways:
- (a) It will commit the land to being used only for purposes of the BMP;
- (b) It will include an agreement by the landowner, to maintain the facilities in accordance with the SMP (this obligation would be passed on to future purchasers or successors of the landowner, as a covenant); and (c) It will include an easement giving the County the right to enter onto the land (and any necessary adjacent land needed for access) to maintain the BMPs.
- This would be required of all applications listed in WPO Section 67.804. In the case of subdivisions, this easement and covenant would be recorded on or prior to the Final or Parcel Map.

ATTACHMENT H CERTIFICATION SHEET

DECLARATION OF RESPONSIBLE CHARGE

I HEREBY DECLARE THAT I AM THE ENGINEER OF WORK FOR THIS PROJECT, THAT I HAVE EXERCISED RESPONSIBLE CHARGE OVER THE DESIGN OF THE PROJECT AS DEFINED IN SECTION 6703 OF THE BUSINESS AND PROFESSIONS CODE, AND THAT THE DESIGN IS CONSISTENT WITH CURRENT STANDARDS.

I UNDERSTAND THAT THE CHECK OF PROJECT DRAWINGS AND SPECIFICATIONS BY THE COUNTY OF SAN DIEGO IS CONFINED TO A REVIEW ONLY AND DOES NOT RELIEVE ME, AS ENGINEER OF WORK, OF MY RESPONSIBILITIES FOR PROJECT DESIGN.

BY: <u>ANNABELLE S. AGUILAR</u>
(PRINT NAME)

RCE NO.: 61158

EXPIRES: <u>06/30/2009</u>

SAN DIEGUITO ENGINEERING, INC. 4407 MANCHESTER AVENUE, SUITE 105 ENCINITAS, CALIFORNIA 92024 PHONE: (760)753-5525



ATTACHMENT I ADDENDUM

ATTACHMENT I - LID DISCUSSION

The following text is from the Municipal Storm Water Permit for San Diego County:

Each Copermittee shall require each Priority Development Project to implement LID BMPs which will collectively minimize directly connected impervious areas and promote infiltration at Priority Development Projects:

(a) The following LID site design BMPs shall be implemented at all Priority Development Projects as required below:

i. For Priority Development Projects with landscaped or other pervious areas, drain a portion of impervious areas (rooftops, parking lots, sidewalks, walkways, patios, etc) into pervious areas prior to discharge to the MS4. The amount of runoff from impervious areas that is to drain to pervious areas shall correspond with the total capacity of the project's pervious areas to infiltrate or treat runoff, taking into consideration the pervious areas' soil conditions, slope, and other pertinent factors.

The project proposes a very low ratio of pervious surface to proposed impervious surface. All proposed drainage from rooftops and roads are directed toward pervious areas.

ii. For Priority Development Projects with landscaped or other pervious areas, properly design and construct the pervious areas to effectively receive and infiltrate or treat runoff from impervious areas, taking into consideration the pervious areas' soil conditions, slope, and other pertinent factors.

Bio-swales located on the proposed pads are 1% slope at locations for post-development runoff. Water is conveyed offsite by existing storm drain pipes to a golf course that serves as a biofilter.

iii. For Priority Development Projects with low traffic areas and appropriate soil conditions, construct a portion of walkways, trails, overflow parking lots, alleys, or other low-traffic areas with permeable surfaces, such as pervious concrete, porous asphalt, unit pavers, and granular materials.

The proposed access road and driveways are constructed to the minimum allowable standards, paving has been minimized.

The below standards have been utilized in the design of this project:

- (b) The following LID BMPs listed below shall be implemented at all Priority Development Projects where applicable and feasible.
 - i. Conserve natural areas, including existing trees, other vegetation, and soils.
 - ii. Construct streets, sidewalks, or parking lot aisles to the minimum widths necessary, provided that public safety and a walkable environment for pedestrians are not compromised.
 - iii. Minimize the impervious footprint of the project.
 - iv. Minimize soil compaction.
 - v. Minimize disturbances to natural drainages (e.g., natural swales, topographic depressions, etc.)